

POKRA 36/98 (11)

9114
OmluBovolo 52

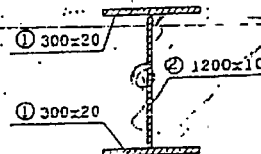
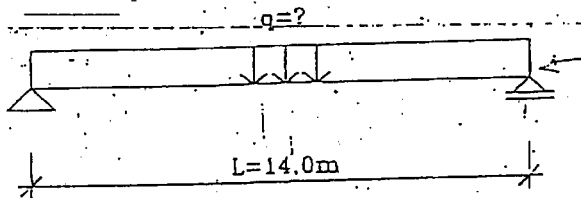
OSNOVE METALNIH KONSTRUKCIJA

PLU: 1+2+3

①+②+③-YPASICIM

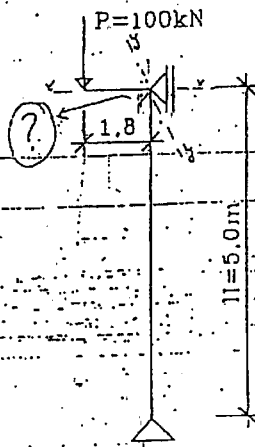
1. ZADATAK

Odrediti maksimalnu vrednost opterećenja q (kN/m) koje može da prenese pun limeni nosač I-preseka formiran zavarivanjem. Nožice nosača su 300×20 , rebro 1200×10 , a podužni ugaoni šavovi za vezi rebra sa nožicama su $a = 3\text{mm}$. Rebro nosača ukrućeno je poprečnim ukrućenjima u desetinama raspona. Nosač je kontinualno bočno pridržan u nivou pritishute nožice. Raspon nosača je $L = 14,0\text{m}$. Osnovni materijal je Č0361. - I slučaj opterećenja, a dopušteni ugib $L/500$.



2. ZADATAK - YPAZIM TUBO

- a) Dimenzionisati stub, prikazan na skici, kao valjani I profil. Pri proračunu smatrati da je stub bočno pridržan na mestima oslonaca.
 - b) Dimenzionisati konzolu i njenu vezu sa stubom u zavarenoj izradi (direktnim zavarivanjem, bez čelone ploče).
- Opterećenje i geometrija nosača prikazani su na skici. Osnovni materijal je Č0361. - II slučaj opterećenja.

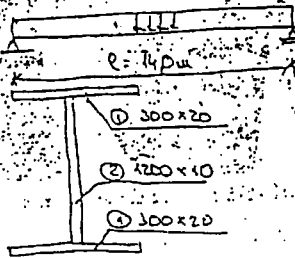


3. ZADATAK

Sračunati i konstruisati u razmeri 1:10 montažni nastavak štapa izrađenog od valjanog profila IPE400 prema površini poprečnog preseka. Predvideti obostrane podvezice na nožicama. Nastavak izvesti pomoću visokovrednih zavrtnećva klase čvrstoće 10.9 sa punom silom pritezanja ($v_1 = 0,8$; $\mu = 0,4$; $v_2 = 1,40$). Osnovni materijal je Č0562 - I slučaj opterećenja.

5m

(14. VII 4997/1)



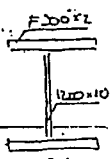
$a = 3 \text{ м}$

- поверхна поверхня на $l' = 1,4 \text{ м}$
- кошик шпално дощ-позркан у чубоу дриш. ноа.

ЄОСГІ - І. а. раалі. \rightarrow $G_{доу} = 16,0 \text{ кН/м}^2$
 $T_{доу} = 9,0 \text{ кН/м}^2$
 $G_{вдоу} = 12,0 \text{ кН/м}^2$

$l_{доу} = \frac{l}{500}$

$H = \frac{g l^2}{2} = 2450 \text{ г}$



$I_x = 530520,0 \text{ см}^4$ $I_{y0} = 446520 \text{ см}^4$
 $A = 240 \text{ см}^2$ $I_{x0} = 144000 \text{ см}^4$
 $W_x = 9524,52 \text{ см}^3$
 $S_x = 5460$

- СРЕДНЯ ПАСПОНА

$H = \frac{g l^2}{8} = 2450,0 \text{ г}$

$\sigma = \frac{H}{W} = \frac{2450,0 \text{ г}}{3524,52} = 16,0 \text{ г} \rightarrow \sigma_1 = 0,672 \text{ кН/см}^2 = 67,2 \text{ кН/м}^2 \checkmark$

$\rho = \frac{5}{384} \frac{g l^4}{E I} = \frac{5}{384} \frac{g \cdot 1400^4}{21103530220} = 4,0326 \text{ г}$

$4,0326 \text{ г} \leq \frac{l}{500} \Rightarrow \sigma_2 = 0,6942 \text{ кН/см}^2 = 69,42 \text{ кН/м}^2 \checkmark$

маб

$V_{II} = \frac{Q \cdot S_x}{I \cdot 2 \cdot 0,2}$; $Q = R = \frac{1400 \cdot g}{2}$

$V_{II} = \frac{1400 \cdot g \cdot 5460}{350520,0 \cdot 2 \cdot 0,2} = 10,7871 \text{ г} \cdot \text{см}$

$G_{II} = \sqrt{V_{II}^2} = 10,7871 \text{ г} < 12 \text{ кН/см}^2$

$\sigma_3 < 1,112 \text{ кН/см}^2 = 111,2 \text{ кН/м}^2 \checkmark$

$\sigma = \max\{\sigma_1, \sigma_2, \sigma_3\} = \max\{67,2; 69,42; 111,2\} = 69,42 \text{ кН/м}^2$

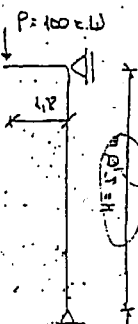
$\sigma = 67,2 \text{ кН/м}^2$

$T = \frac{g l^2 / 2 \cdot S}{I \cdot \rho_{доу}} \leq T_{доу} \checkmark$

(14. JUL. 1997, ЗДА. 2)

305
669
ср.

от 3



а) димензионални I, доколку се изврши по користна остатоци

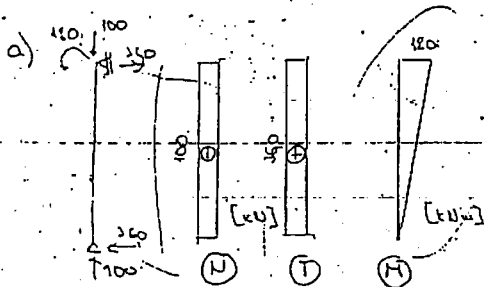
б) конзола директно заварета

СВЗГ, II сур. отпор.

$\sigma_{доп} = 18 \text{ кН/см}^2$

$\sigma_v = 240 \text{ МПа}$

$\tau_{доп}$

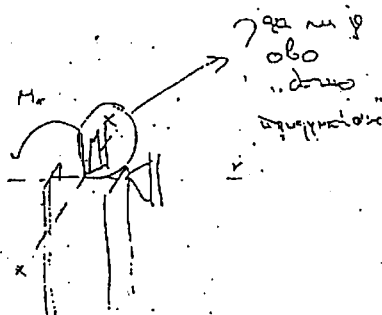


- димензионален е со брзи за пресек на брзи ситуација

$M = P \cdot a = 100 \text{ кН} \cdot 1.8 \text{ м} = 180 \text{ кНм}$

$R = \frac{M}{W} = \frac{180}{0.5} = 360 \text{ кН}$

$W = \frac{M}{\sigma_{доп}}$



р. 318
5.2

а.1) УБОР ПОПРЕЧНОГ ПРЕСЕКА

продолжителност I 400

$A = 118 \text{ см}^2; W_x = 1460 \text{ см}^3; i_x = 15.7; I_x = 25710 \text{ см}^4; I_D = 40 \text{ см}^4$

$S_x = 851 \text{ см}^3; W_y = 149 \text{ см}^3; i_y = 3.13; I_y = 1160 \text{ см}^4$

$\max(\sigma) = \sigma_{Hx} + \sigma_v = \frac{180 \cdot 10^2}{1460} + \frac{100}{118} = 13.18 \text{ кН/см}^2 < \sigma_{доп} = 18.0 \text{ кН/см}^2$

$\sigma_{Hx} + \sigma_v < \sigma_{доп}$

$\sigma_v = \frac{M}{W} \quad \sigma_{Hx} = \frac{M x}{W_x}$

- величине y равни уравној на $x-x$ осу

$l_x = l_y = 500 \text{ см}$

$\lambda_x = \frac{l_x}{i_x} = \frac{500}{15.7} = 31.85$

$\bar{\lambda}_x = \frac{\lambda_x}{\lambda_v} = \frac{31.85}{92.8} = 0.34$

- крива изборна "А" (за $x-x$), $\alpha = 0.200$

- Витростан y ($y-y$)

$$\lambda_y = \frac{500}{3,10} = 159,74$$

$$\bar{\lambda}_y = \frac{159,74}{92,9} = 1,72$$

- пружа слабостан (B'' ($y-y$), $\alpha = 0,333$)

$$k_{nx(y)} = 1 + \frac{\alpha(\bar{\lambda}_{x(y)} - 0,2)}{1 - \bar{\lambda}_{x(y)}^2 \cdot \bar{c}} ; \quad \bar{c} = \gamma \frac{G_U}{G_V} = 1,23 \cdot \frac{0,847}{24} = 0,047$$

$$k_{nx} = 1 + \frac{0,206(0,34 - 0,2)}{1 - 0,34^2 \cdot 0,047} = 1,029$$

$$k_{ny} = 1,6$$

$$k_{max} = \frac{\lambda_x}{1 - \bar{\lambda}_x^2 \cdot \bar{c}} = \frac{0,66}{1 - 0,34^2 \cdot 0,047} = 0,6636$$

$$Q = \frac{G_V}{G_d} \geq 1,0$$

- JUS 0. E7. 101, таблица 4, смрт. ст. (норми!)

$$G_d = \lambda_p \cdot G_V \cdot \lambda_H \leq G_V$$

$$\lambda_p = \frac{W_{xpl}}{W_x} = \frac{2 S_x}{W_x} = \frac{2 \cdot 853,0}{1460} = 1,174$$

$$\lambda_H) \bar{\lambda}_d = \sqrt{\lambda_p \frac{G_V}{G_{cr,d}}}$$

$$G_{cr,d} = \phi \cdot \sqrt{G_{ud}^2 + G_{wd}^2}$$

$$\phi = \frac{\sqrt{k + p^2} + p}{\sqrt{k + p^2}}$$

- таблица 2, смрт. ст. $\Rightarrow p = 0 \Rightarrow \phi = 1$

$$\Rightarrow G_{ud} = \eta_t \frac{\pi}{l_t \cdot W_x} \sqrt{G \cdot E \cdot J_d \cdot I_y} = \eta_t \frac{0,91 \cdot 10^5}{l_t \cdot W_x} \sqrt{J_d \cdot J_y}$$

$$l_t = 500 \text{ cm}; \quad \text{смрт. ст.}, \alpha \Rightarrow \eta_t = 1,77$$

$$J_d = \frac{1}{3} (2 \cdot b_p \cdot t_p^3 + h_w \cdot t_w^3) =$$

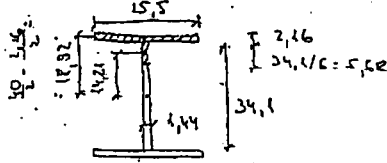
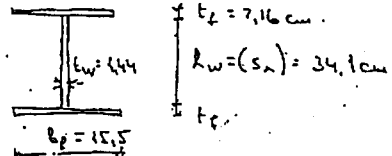
$$= \frac{1}{3} (2 \cdot 15,5 \cdot 2,16^3 + 34,1 \cdot 1,14^3) = 138,08 \text{ cm}^4$$

$$G_{ud} = 1,77 \cdot \frac{0,91 \cdot 10^5}{500 \cdot 1460} \sqrt{138,08 \cdot 1160} = 39,78 \frac{\text{N}}{\text{cm}^2} \quad (G_{ud} = 44,15, \text{ за } J_d = 120)$$

$$\rightarrow G_{wd} = \frac{\pi^2 \cdot E}{\lambda^2 t_y} = \frac{2,02 \cdot 10^5}{\lambda^2 t_y}$$

$$\rightarrow \lambda_{ky} = \frac{l_y}{t_y \sqrt{\eta_y}} ; \quad \eta_y = \frac{G_{cr,y}}{G_{cr,x}} = 1,77$$

$$l_y = 500 \text{ (дочно приуцнан на касишна основанца)}$$



ИЧОТ 1994

ПРЧ 00

VV 10.9

$\lambda = 0.8$

$\mu = 0.4$

$\gamma = 1.10$

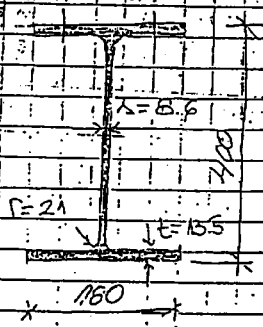
0.0582

I бича от желез

$b_{01} = 24$ cm

$b_{02} = 90$ cm

$\times 36$



$A = 84.5 \text{ cm}^2$

погезије на томици

погезије на томици

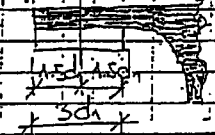
$2b_{max} \leq b = 18 - 2 \cdot 1 = 16 - 0.86 = 15.14 \text{ cm}$

$b_{01}, b_{02} \leq 6.47 \text{ cm}$

$d_1 \leq \frac{b_{max}}{3}$

$d_1 \leq \frac{6.4}{3}$

$d_1 \leq 2.13 \text{ cm}$



$96 - \lambda - 2\gamma = 3d_1$

$d_1 = \frac{96 - 0.86 - 2 \cdot 2.1}{3}$

$d_1 = 1.51 \text{ cm}$

учејето VV 10.9

$\frac{A_{нот}}{A_{нот, макс}} \geq \frac{A_{нот}}{A_{нот, макс}}$

$[(b_{max} - 2d_1) + 2 \cdot (b_{max} - d_1)] \cdot t_{нот} \geq (b - 2d_1) \cdot t$

$t_{нот} \geq \frac{(b - 2d_1) \cdot t}{[(b_{max} - 2d_1) + 2 \cdot (b_{max} - d_1)]} = \frac{(18 - 2 \cdot 2.1) \cdot 1.35}{[(18 - 2 \cdot 2.1) + 2 \cdot (6.4 - 2.1)]} = 0.83 \text{ cm}$

$t_{нот} = 10 \text{ mm}$

учејето погезије на томици $\neq 180 \times 10 + 2 \neq 64 \times 10$

Обол. буага һа формула:

$$F_p = \mu \cdot \sigma_{0.2} \cdot A_s$$

$$F_{pc} = \mu \cdot \frac{F}{\gamma_2} = \mu \cdot \frac{\sigma_{0.2} \cdot A_s}{\gamma_2} = 0.4 \cdot \frac{0.8 \cdot 30 \cdot 2.45}{1.40} = 50.40 \text{ kN}$$

$$n_{\text{max}} = \frac{A_{\text{нето}}}{A_{\text{max}} \cdot \sigma_{\text{доп}}} = \frac{20.1 \cdot 24}{50.4} = 9.57 \text{ ком}$$

$F_{pc} = 30 \text{ тл}$ $\text{учбетено: } 10 \text{ М}20 \times 6 \dots 10.9$

$$A_{\text{max}} = b \cdot l = 18 \cdot 1.35 = 24.3 \text{ см}^2$$

$$A_{\text{нето}} = A_{\text{max}} - 2d_1 = 24.3 - 2 \cdot 2.1 = 20.1 \text{ см}^2$$

$$(l_{\text{min}} d_1 - 15 t_s \text{ min} - 0.2) = 15 \cdot 1.0 - 0.2 = 14.8 \text{ см}$$

Уогбезуца һа редня

$$l_{\text{max}} d_1 = \sqrt{15 t_s \text{ min} - 0.2} = \sqrt{15 \cdot 0.8 - 0.2} = 1.8 \text{ см}$$

Учбетено бу: бб: буцуи: М16 x 6 ... 10.9

$$l_{\text{max}} \leq h - 2c = 33.1 \text{ см} \quad \text{учб: } h_{\text{рас}} = 30 \text{ см}$$

$$2 \cdot (l_{\text{рас}} - 4d_1) \cdot \frac{F_{pc}}{l_{\text{рас}}} \geq A_{\text{нето}}$$

$$\frac{F_{pc}}{l_{\text{рас}}} \geq \frac{A - 2A_{\text{max}} - 4d_1 \cdot s}{2 \cdot (l_{\text{рас}} - 4d_1)}$$

$$\frac{50.4}{l_{\text{рас}}} \geq \frac{84.5 - 2 \cdot 24.3 - 4 \cdot 1.7 \cdot 0.86}{2 \cdot (30 - 4 \cdot 1.7)} = 0.65 \text{ см}$$

$$l_{\text{рас}} = 8 \text{ см}$$

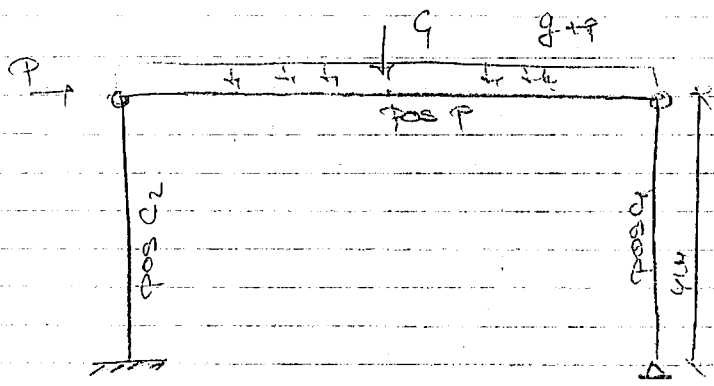
Учбетено уогбезуца һа редня 2 x 300 x 0.8

Учбетено бу: буага һа редня

$$n_{\text{рас}} = \frac{A_{\text{нето}} \cdot \sigma_{\text{доп}}}{F_{pc}} = \frac{A - 2A_{\text{max}} - 4d_1 \cdot s}{F_{pc}} = \frac{29.1 \cdot 24}{32.3} = 21.6$$

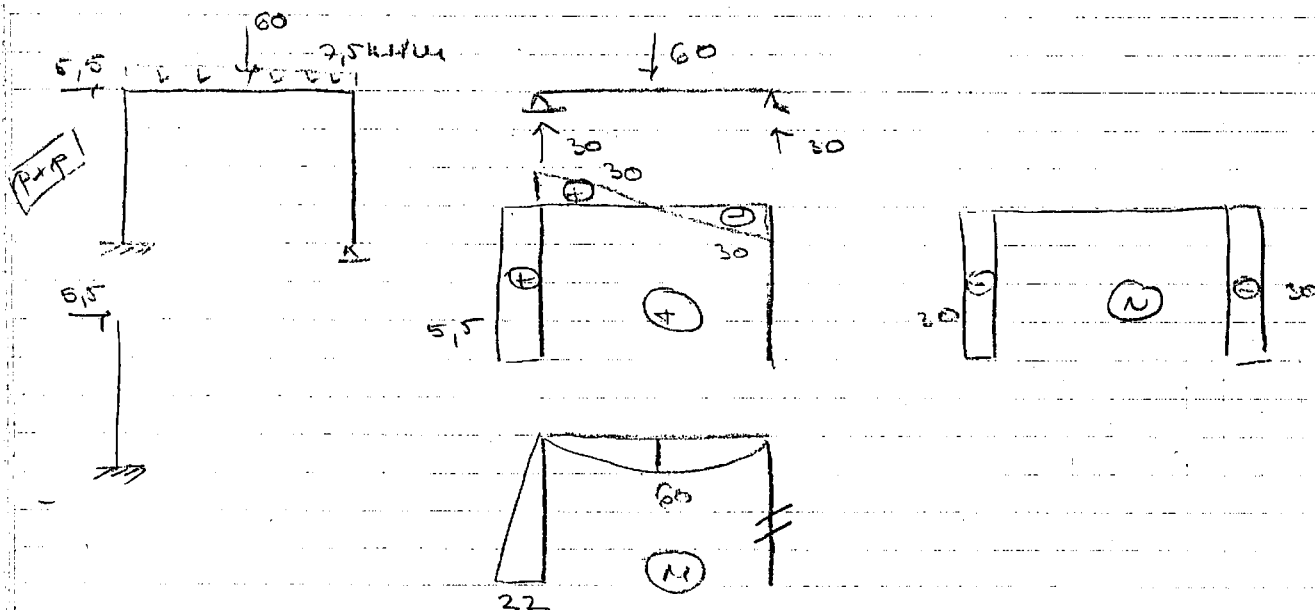
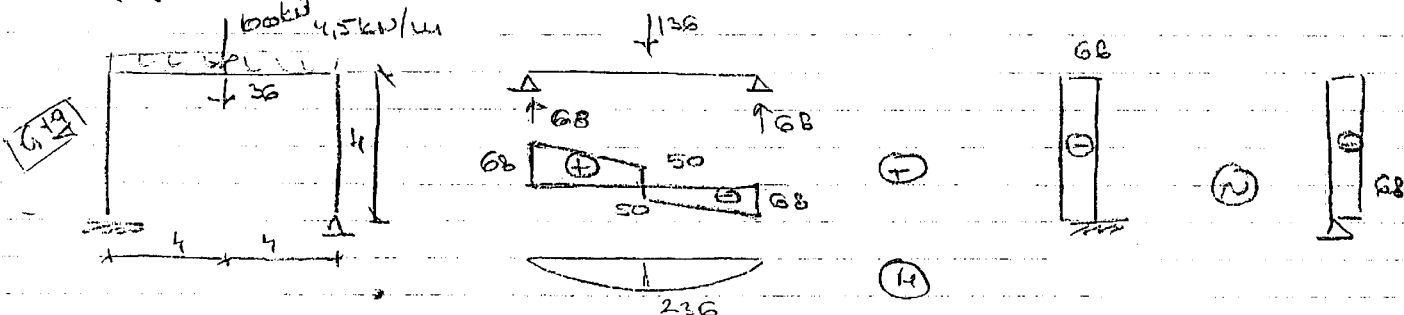
$$F_{pc} = \mu \cdot \frac{F}{\gamma_2} = 0.4 \cdot \frac{0.8 \cdot 50 \cdot 1.57}{1.40} = 32.3 \text{ kN}$$

$$A_s (\text{М}16) = 1.57 \text{ см}^2 \quad n_{\text{рас}} = \frac{29.1 \cdot 24}{32.3} = 21.6$$



a) dimensioniranje nosača P prema JUS-u, koristeći načelo najmanje potrošnje materijala; $f_{dop} = 2,7 \text{ cm}$

-dijagrami presjernih sila



nos P

$f_{dop} = 2,7 \text{ cm}$

$$W_{pot} = \frac{M_{max}}{\sigma_{dop}} = \frac{(236 + 68) \cdot 100}{16} = 1850 \text{ cm}^3$$

• odabir se HEA 450

- $h = 440$
- $b = 300$
- $t_w = 11,5$
- $t_f = 21$

- $A = 128$
- $I_x = 63220$
- $W_y = 2500$
- $i_y = 18,9$

- $I_y = 9470$
- $S_y = 1610$
- $W_x = 631$
- $i_x = 7,29$

$$f_{max1} = \frac{5}{384} \cdot \frac{\left(\frac{4,5 \cdot 7,7}{100}\right) \frac{\text{KN}}{\text{cm}^2} \cdot (800 \text{ cm})^4}{21000 \cdot 63720} = 0,48 \text{ cm}$$

$$f_{max2} = \frac{F \cdot l^3}{48EI} = \frac{100 \cdot 800^3}{48 \cdot 21000 \cdot 63720} = 0,8$$

$$f_{max} = 1,28 \text{ cm}$$

* KONTROLA NAPONA

$$\sigma_{max} = \frac{M_{max}}{W_y} = \frac{29600}{2900} = 10,2 \text{ kN/cm}^2 \leq 16 \text{ kN/cm}^2$$

$$T_{max} = \frac{T_{max} S_y}{I_y \cdot t_w} = \frac{(30+60) \cdot 1010}{63720 \cdot 11,5} = 2,153 \text{ kN/cm}^2 \leq 9 \text{ kN/cm}^2$$

* KONTROLA UPOREDNOG NAPONA NA MESTU MAX. MOMENTA
 Tady je transverzalna sila na tom mestu
 0 u tom poprečnom preseku

$$\sigma_1 = \frac{M_{max}}{I_y} \cdot \frac{h - 2t_f}{2} = \frac{29600}{63720} \cdot \frac{44 - 2 \cdot 21}{2} = 9,24$$

$$S_{y,0} = b \cdot t_f \cdot \frac{h - t_f}{2} = 1319,85$$

$$t_{1,0} = \frac{T_{max} \cdot S_{y,0}}{t_w \cdot I_y} = \frac{50 \cdot 1319,85}{11,5 \cdot 63720} = 0,9 \text{ kN/cm}^2$$

$$\sigma_u = \sqrt{\sigma_1^2 + 3t_{1,0}^2} = \sqrt{9,24^2 + 3 \cdot 0,9^2} = 9,37 < 16 \text{ kN/cm}^2$$

b) KONTROLA NABUČENJA NA SAUJAZU I SNIČENJE PLEHA EC3

$$M_{ed} = 1,35 \cdot 230 + 1,5 \cdot 60 = 408,6 \text{ kNm}$$

$$V_{ed} = 1,35 \cdot 68 + 1,5 \cdot 30 = 136,8 \text{ kN}$$

S235 = 1 E = 1 KLASIFIKACIJA POP. PROSEKA

• nožica: $\frac{(b - t_w - 2 \cdot r)}{2 \cdot t_f} =$

• rebro: $\frac{(h - 2 \cdot t_f - 2 \cdot r)}{t_f} =$

$$\frac{W_{pl,y1}}{S_{y,0}} = \frac{3215 \cdot 23,5}{1} = 7552,7$$

$$\frac{860}{52,5} = 0,54 < 1$$

→ POBILAN SURKALAN

$$A_v = A - 2 \cdot b \cdot t_f + (t_w + 2t_f) t_f = 128 - 2 \cdot 30 \cdot 2,1 + (1,15 + 5,4) \cdot 2,1 = 65,75$$

$$U_{pe,rd} = A_v \cdot \frac{(r_y / \sqrt{I_x})}{\sqrt{3}} = \frac{65,755 \cdot 23,5}{\sqrt{3}} = 892,146$$

$$U_{Ed} / U_{pe,rd} = \frac{136,8}{892,146} = 0,152 < 1$$

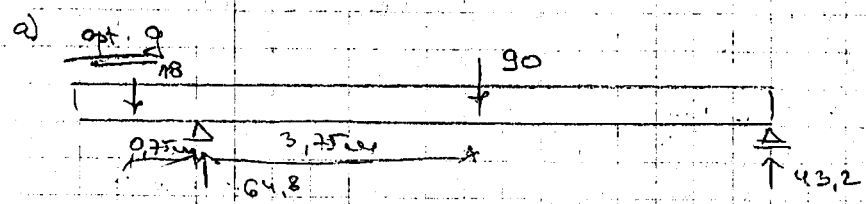
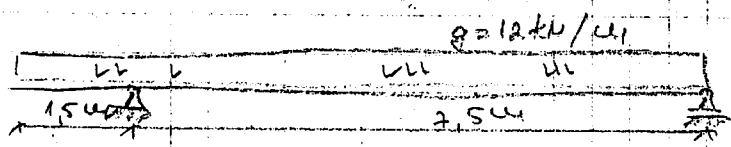
$U_{Ed} < 0,8 U_{pe,rd}$ ✓ - No moraciona vrsta redukcija nosivosti na nosivost surkalan

v) slab nos C_1 , bova puzanja na kugulana
centratano puzant (2. vaba)

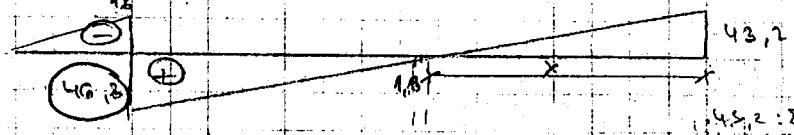
ii) slab nos C_2 , bova puzanja na kugulana
ekcentratano puzant (6. vaba)

bova tabula
1200/120

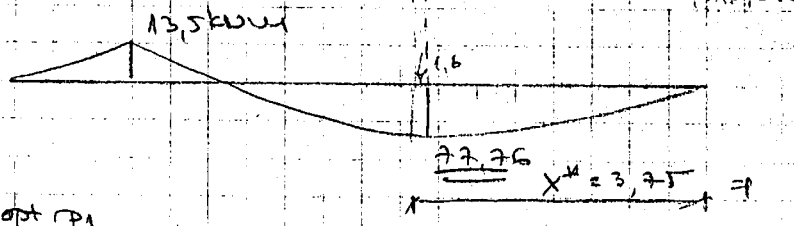
① $q = 12 \text{ kN/m}$
 $p = 15 \text{ kN/m}$
 $r = 15 \text{ kN/m}$



(T)



(M)



$$43.2 \cdot x = 46.8 \cdot (7.5 - x)$$

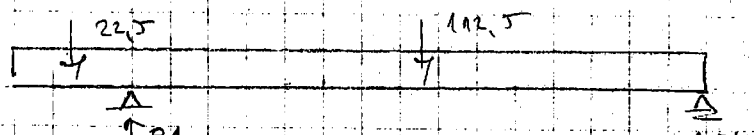
$$43.2x = 35.1 - 46.8x$$

$$x = 3.75 \text{ m}$$

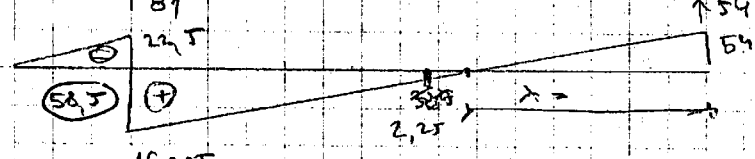
$$3 \cdot 0 \cdot 43.2 - 12 \cdot 3.75 \cdot \frac{3.75}{2} = 77.76$$

$$M_{\text{right}} = 43.2 \cdot 7.5 - 12 \cdot \frac{7.5^2}{2} = 77.76 \text{ kNm}$$

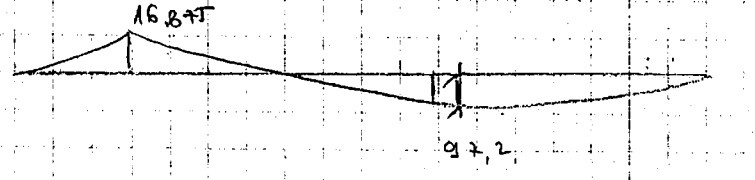
opt P1



(T)



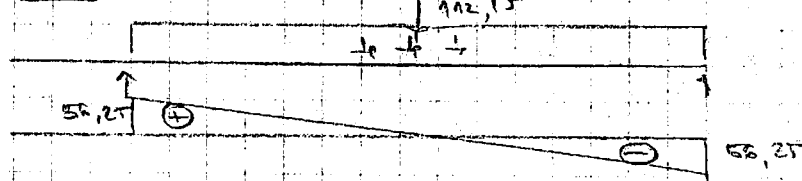
(M)



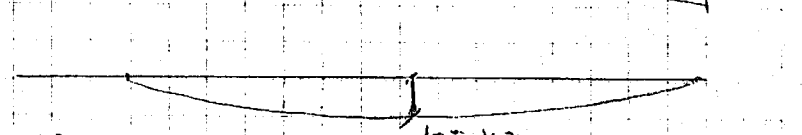
$$58.5 \cdot x = 2.25 \cdot 54 - 54 \cdot x$$

$$x = 2.25$$

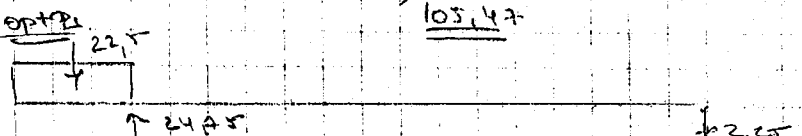
opt P2



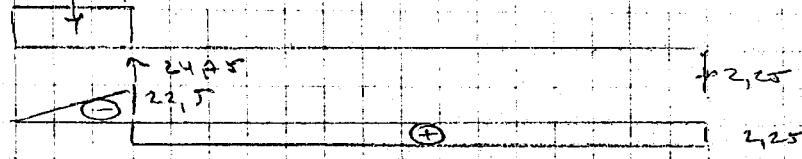
(T)



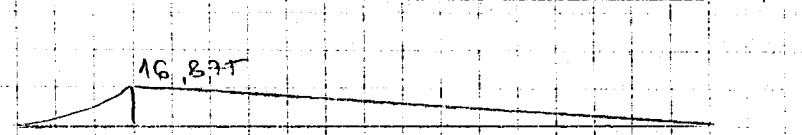
(M)



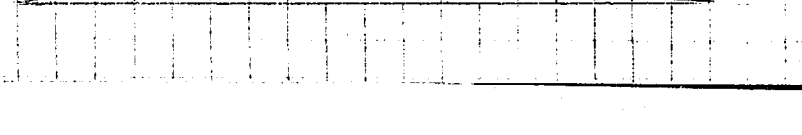
opt P3



(T)



(M)



a) IUS Dimensionierung
- kontinuierlich beanspruchtes Problem

$$\begin{matrix} I_{PE} \\ 3235 \\ \text{Isl. opt} \end{matrix} \left. \begin{matrix} \sigma_d = 10 \text{ kN/cm}^2 \\ \tau_d = 9 \text{ kN/cm}^2 \end{matrix} \right\} \begin{matrix} \text{KMP} \\ \text{f}_{dop} = 1 \text{ cm} \\ \text{f}_{dop} = 2,5 \text{ cm} \end{matrix}$$

$$W_{pot} = \frac{M_{max}}{\sigma_{dop}} \quad M_{max} = 27,02 + 105,47 = 132,49 \text{ kNm}$$

$$W_{pot} = \frac{132,49 \cdot 100}{10} = 1324,9 \text{ cm}^3$$

➔ Auswahl IPE 500

$h = 500 \text{ mm}$	$A = 118 \text{ cm}^2$		
$b_f = 200 \text{ mm}$	$I_y = 48200 \text{ cm}^4$	$I_z = 2940 \text{ cm}^4$	$S_y = 1100$
$t_w = 10,2 \text{ mm}$	$W_y = 1930 \text{ cm}^3$	$W_z = 214 \text{ cm}^3$	
$t_f = 18 \text{ mm}$	$i_y = 20,4 \text{ cm}$	$i_z = 4,31 \text{ cm}$	
$r = 29 \text{ mm}$			

➔ Kontrolle Normale

$$\sigma_{max} = \frac{M_{max}}{W_y} = \frac{132,49 \cdot 100}{1930} = 6,86 \text{ kN/cm}^2 < \sigma_d \quad W$$

$$\tau_{max} = \frac{T_{max} \cdot S_y}{t_w \cdot I_y} = \frac{(58,5 + 46,8) \cdot 100}{10,2 \cdot 48200} = 2,35 \text{ kN/cm}^2 < \tau_d \quad W$$

➔ Kontrolle Ufereinander Normale (na mesto udeležene momenta enajstev)

$$\sigma_1 = \sqrt{\sigma_1^2 + 3\tau^2}$$

$$\sigma_1 = \frac{M_{max}}{I_y} \cdot \frac{h - 2 \cdot t_f}{2} = \frac{132,49 \cdot 100}{48200} \cdot \frac{(50 - 2 \cdot 18)}{2} = 8,88 \text{ kN/cm}^2 < \sigma_{dop}$$

$$\tau_1 = \frac{T_{max} \cdot S_{y0}}{t_w \cdot I_y} = \frac{[T_{max} - 3,75 \cdot (q + p)] \cdot S_{y0}}{t_w \cdot I_y}$$

$$S_{y0} = b_f \cdot t_f \cdot \frac{h - t_f}{2} = 20 \cdot 18 \cdot \frac{50 - 18}{2} = 774 \text{ cm}^3$$

$$\tau_1 = \frac{(58,5 + 46,8 - 3,75 \cdot 37) \cdot 774}{10,2 \cdot 48200} = 0,064 \text{ kN/cm}^2 < \tau_{dop} \quad W$$

$$\sigma_0 = \sqrt{8,88^2 + 3 \cdot 0,064^2} = 8,89 \text{ kN/cm}^2 < \sigma_{dop} = 10 \text{ kN/cm}^2 \quad W$$

b) KONTROLA GRANICNIH STANJA NOSIVOSTI PREMA EC3.

o uticaj na uskl. uskl. momenta

$$M_{ed} = 1,35 \cdot 77,62 + 1,5 \cdot 105,47 = 262,992 \text{ kNm}$$

$$V_{ed} = 1,35 \cdot 1,8 + 1,5 \cdot 2,25 = 5,805 \text{ kN}$$

o uticaj nad osloncem

$$V_{ed} = 1,35 \cdot 46,8 + 1,5 \cdot 58,5 = 150,93$$

* KLASIFIKACIJA POPREČNOG PRESEKA

osnovni materijal S235 $\alpha = 1$

o nosica:
$$\frac{(b_p - t_w - 2 \cdot r)}{t_f} = \frac{(20 - 1,02 - 2 \cdot 2,1)}{1,8} = 4,62$$

o rebar:
$$\frac{(h - 2t_f - 2 \cdot r)}{t_f} = \frac{(50 - 2 \cdot 1,8 - 2 \cdot 2,1)}{1,6} = 26,625$$

* NOSIVOST POPREČNOG PRESEKA NA DEJSTVO SMICUJUĆIH SILA

- POUKAZIVA SMIČANJA

$$A_v = h - 2 \cdot b \cdot t_f + (t_w + 2r)t_f = 118 - 2 \cdot 20 \cdot 1,8 + (1,02 + 2 \cdot 2,1) \cdot 1,6 = 62,64$$

$$V_{pl,rd} = \frac{A_v \cdot (f_t / \sqrt{3})}{\gamma_{M0}} = \frac{62,64 \cdot (23,5)}{1,05} = 849,88 \text{ kN}$$

* OSLOMAČENI PRISTEK

$$V_{ed} / V_{pl,rd} = 150,93 / 849,88 = 0,177 < 1$$

* INTERAKCIJA SMICUJANJA I SMIČANJA [P7-35]

$$V_{ed} = 5,805 \text{ kN} < 0,5 \cdot V_{pl,rd} = 0,5 \cdot 849,88 \text{ kN}$$

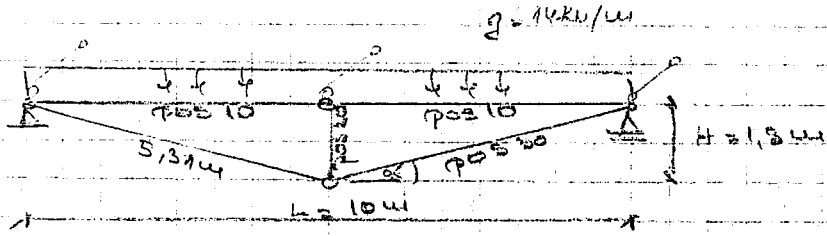
ako smičuće sile ne iznose više od 50%
na unutrašnje uskl. redukciju nosivosti
na momente savijanja

* KONTROLA USIBA, KONTROLA GRANICNIH STANJA UPOTREBLJIVOSTI

$$f_{max}^{pog} = \frac{5}{384} \cdot \frac{(q + p) \cdot l^4}{I_y \cdot E} = \frac{5}{384} \cdot \frac{(15 + 0,2) \cdot 950^4}{48 \cdot 200 \cdot 21000} = 1,10 \text{ cm} < 2,5 \text{ cm}$$

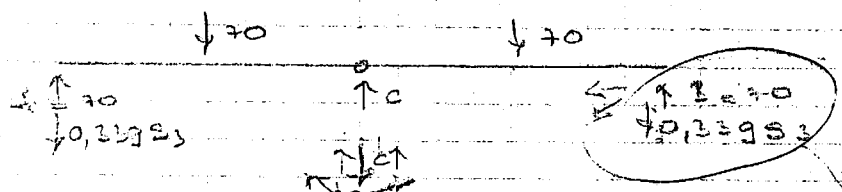
$$f_{max}^{prop} = \frac{(q + p) \cdot l^3}{24EI} (3 \cdot 150^3 - 750^3 + 4 \cdot 150^2 \cdot 750) = -0,57 \text{ cm} < 1 \text{ cm}$$

2



$\alpha = 13,799$
 $\sin \alpha = 0,339$

a) HEA, 335, I 36, JUS



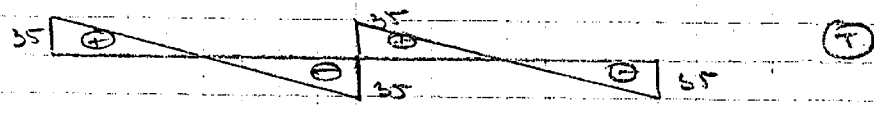
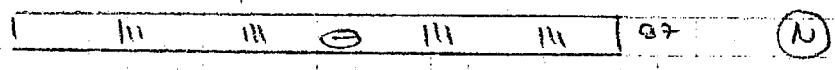
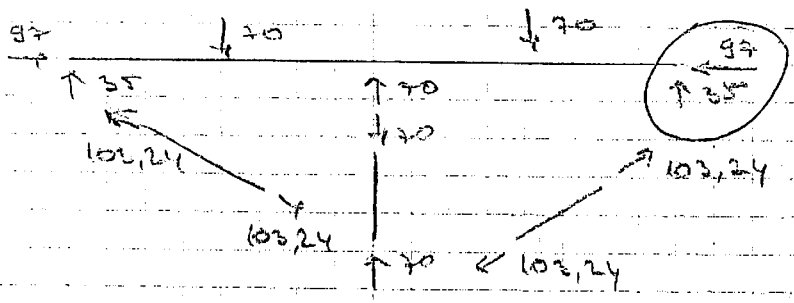
$M_{C=0} : 70 \cdot 2,5 - 2 \cdot 5 + 0,339 \cdot 24 \cdot 5 = -175 + 0,339 \cdot 24 \cdot 5 = 0$

$C = 2 \cdot 0,339 \cdot 24 = A + C = 110$
 $A = H = 70$

$S_3 = \frac{125}{5 \cdot 0,339}$

$S_3 = 103,24$

$C = 70$



Poslo

$335 \left\{ \begin{array}{l} \sigma_d = 24 \text{ kN/cm}^2 \\ \tau_d = 14 \text{ kN/cm}^2 \end{array} \right.$

$W_{pot} = \frac{W_{max}}{\sigma_d} = \frac{43,75 \cdot 100}{24} = 182,29 \text{ cm}^3$

$A_{pot} = \frac{N_c}{\sigma_d} = \frac{97}{24} = 4,04 \text{ cm}^2$

* Uvagaňo HEA 200

$h = 110 \text{ mm}$
 $b_f = 200 \text{ mm}$
 $s = t_w = 6,5 \text{ mm}$
 $t_f = 10 \text{ mm}$
 $r = 18 \text{ mm}$

$A = 53,8 \text{ cm}^2$

$I_y = 3890 \text{ cm}^4$

$W_y = 389 \text{ cm}^3$

$I_y = 8,28 \text{ cm}$

$I_z = 1340 \text{ cm}^4$

$W_z = 134 \text{ cm}^3$

$I_z = 4,92 \text{ cm}$

$S_y = 215 \text{ cm}^3$

$W_{y,pl} = 430 \text{ cm}^3$

$\sigma_{max} = \frac{N}{A} + \frac{M_{max}}{W_y} = \frac{97}{53,8} + \frac{43,75 \cdot 100}{389} = 13,05 < 24$

$\tau_{max} = \frac{T_{max} \cdot S_y}{t_w \cdot I_y} = \frac{85 \cdot 215}{0,65 \cdot 3890} = 3,14 < 14$

$\rho_{max} = \frac{1}{384} \frac{5 \cdot l^4}{E \cdot I_y} = \frac{5}{384} \cdot \frac{0,1^4 \cdot 500^4}{21000 \cdot 3890} = 1,47 \text{ cm}$ (gledano polovine)

* KONTROLA EKSCENTRIČNO PRITISNUTOG ELEMENTA

$\left| k_n \cdot \sigma_n + \theta_{kny} \cdot \sigma_{My} \right| \leq \sigma_{dop}$

$\sigma_n = \frac{N}{A} = \frac{97}{53,8} = 1,8$

$\sigma_{My} = \frac{M}{W_y} = \frac{43,75 \cdot 100}{389} = 11,25$

$\frac{\sigma_n}{\sigma_{dop}} = \frac{1,8}{24} = 0,075$

outkost ($e_{1y} = e_{1z}$ jer razmatramo pol. nosača, u opštem slučaju $e_{1y} = 1 \cdot e$, a e_{1z} glednu u zavisnosti toga je bilo proračun + elaborat)

~ u osi y: $\lambda_y = \frac{e_{1y}}{I_y} = \frac{500}{8,28} = 60,38$

~ u osi z: $\lambda_z = \frac{e_{1z}}{I_z} = \frac{500}{4,92} = 101,63$

$\bar{\lambda}_y = \frac{\lambda_y}{\lambda_1} = \frac{60,38}{76,4} = 0,79$

$\bar{\lambda}_z = \frac{\lambda_z}{\lambda_1} = 1,33$

$\lambda_1 = 76,4$ (S355)

$\lambda_1 = 86,81$ (S275)

$\lambda_1 = 92,9$ (S235)

- koeficijent imperfekcije

$\alpha_y = 0,206$ (A)

$\alpha_z = 0,329$ (B)

$\beta_y = 1 + \alpha_y (\bar{\lambda}_y - 0,2) + \bar{\lambda}_y^2 = 1 + 0,206(0,79 - 0,2) + 0,79^2 = 1,746$

$\beta_z = 1 + \alpha_z (\bar{\lambda}_z - 0,2) + \bar{\lambda}_z^2 = 1 + 0,329(1,33 - 0,2) + 1,33^2 = 3,152$

$\eta_y = \frac{2}{\beta_y + \sqrt{\beta_y^2 - 4 \cdot \bar{\lambda}_y^2}} = \frac{2}{1,746 + \sqrt{1,746^2 - 4 \cdot 0,79^2}} = 0,804$

$\eta_z = \frac{2}{\beta_z + \sqrt{\beta_z^2 - 4 \cdot \bar{\lambda}_z^2}} = \frac{2}{3,152 + \sqrt{3,152^2 - 4 \cdot 1,33^2}} = 0,413$

$$k_{ny} = 1 + \frac{\mu_y (\bar{\lambda}_y - 0,2)}{1 - \bar{\lambda}_y^2 \cdot \bar{\sigma}_y} = 1 + \frac{0,206 (0,79 - 0,2)}{1 - 0,79^2 \cdot 0,095} = 1,127 < \frac{1}{\bar{\lambda}_y} = 1,244$$

$$k_{nz} = 1 + \frac{\mu_z (\bar{\lambda}_z - 0,2)}{1 - \bar{\lambda}_z^2 \cdot \bar{\sigma}_z} = 1 + \frac{0,339 (1,33 - 0,2)}{1 - 1,33^2 \cdot 0,095} = 1,442 < \frac{1}{\bar{\lambda}_z} = 2,421$$

$$k_n = \max \{ k_{ny}, k_{nz} \} = \frac{1,442}{1} \quad \boxed{1 \leq k_n \leq \frac{1}{\bar{\lambda}_z}}$$

$$k_{wy} = \frac{\beta_y}{1 - \bar{\lambda}_y^2 \cdot \bar{\sigma}_y} = \frac{1}{1 - 0,79^2 \cdot 0,095} = 1,049 \approx 1 = \boxed{k_{wy} = 1,049}$$

STABILNOST NOŽEV NA ZGORNJEM KROVNEM RAVNANJU

$$\bar{\lambda}_z = \frac{l_z}{i_z} = \frac{500}{\frac{20}{\sqrt{12}}} = 86,60 \approx 40 \sqrt{\frac{23,5}{50}} \Rightarrow \text{KRS SE KONTROLA NA 2. r. 12. 678. stranu}$$

- računski izračun nos. pričetkov (delovnih)

poizvedek N. dela krogla zaradi razporeditve nosilcev i 1/6 dolžina

$$A_f = b_f \cdot t_f = 20 \cdot 1 = 20 \text{ cm}^2$$

$$A_w = A - 2 \cdot A_f = 53,8 - 2 \cdot 20 = 13,8 \text{ cm}^2$$

$$i_{kt} = \frac{b_f}{\sqrt{12}} \sqrt{\frac{A_f}{A_f + A_w}} = \frac{20}{\sqrt{12}} \sqrt{\frac{20}{20 + 13,8}} = 5,47 \text{ cm}$$

- Se uverjava kritični napon = 0

$$\sigma_{00} = \eta_t \cdot \frac{0,41 \cdot 10^5}{l_t \cdot W_y} \sqrt{|I_z \cdot I_t|} = 1,12 \cdot \frac{0,41 \cdot 10^5}{500 \cdot 389} \sqrt{1340 \cdot 21,1} = 39,7 \text{ kN/cm}^2$$

- kritični napon določimo

$$\bar{\lambda}_{kz} = \frac{l_z}{\sqrt{\mu_z \cdot i_{kt}}} = \frac{500}{\sqrt{1,12 \cdot 5,47}} = 86,37$$

$$\sigma_{0w} = \bar{\lambda}_{kz}^2 \cdot \frac{E}{I_z} = \frac{2,07 \cdot 10^5}{86,37^2} = 27,75 \text{ kN/cm}^2$$

- kritični napon bočnega-torzijskega izprijanja

$$k = 1 + 0,156 \left(\frac{l_z}{h} \right)^2 \cdot \frac{I_t}{I_z} = 1 + 0,156 \left(\frac{500}{19} \right)^2 \cdot \frac{21,1}{1340} = 2,70$$

$$\phi = \frac{\sqrt{k + p^2} - p}{\sqrt{k + p^2}} = \frac{\sqrt{2,70 + 0,46^2} - 0,46}{\sqrt{2,70 + 0,46^2}} = 0,93$$

$$\sigma_{0z} = \phi \sqrt{\sigma_{00}^2 + \sigma_{0w}^2} = 0,93 \sqrt{39,7^2 + 27,75^2} = 35,36$$

$$\lambda_p = \frac{2 \cdot S_y}{W_y} = 1,105$$

- relativna bodimentoznost otkosa $\bar{\lambda}_0 = \sqrt{\lambda_p \cdot \frac{E}{\sigma_{0z}}} = \sqrt{1,105 \cdot \frac{35,5}{35,36}} = 1,053$

- bodimentoznost kof. bočnega-torzijskega izprijanja $n=2$ za valjasto prosto

$$f_{0z} = \left(\frac{1}{1 + \bar{\lambda}_z^2 \cdot n} \right)^{1,0} = \left(\frac{1}{1 + 1,053^2 \cdot 4} \right)^{1/2} = 0,67$$

- granicu nosnosti bezobrazovnog stupnja

$$\sigma_D = \sigma_{p, \text{dop}} \cdot \eta_y \leq R_y \Rightarrow \sigma_D = 1,105 \cdot 0,67 \cdot 35,5 \leq 35,5 \quad \checkmark$$
$$\sigma_D = \underline{26,28 \text{ kN/cm}^2}$$

$$\sigma_{p, \text{dop}} = \frac{\sigma_D}{\eta_y} = \frac{26,28}{1,5} = \underline{17,52 \text{ kN/cm}^2}$$

$$\theta = \frac{h_y}{\sigma_D} = \frac{1}{\sigma_{p, \text{dop}}} \geq 1 \quad ; \quad \theta = \frac{35,5}{26,28} = 1,35$$

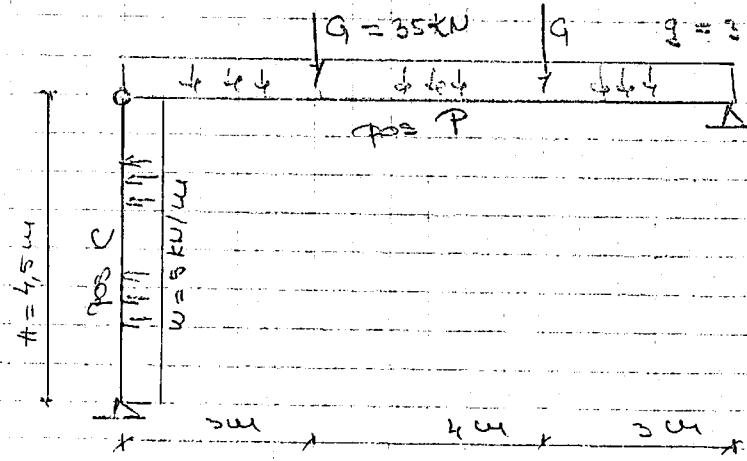
* KONTROLA STABILNOSTI

$$1,442 \cdot 1,8 + 1,35 \cdot 1,049 \cdot 11,25 \leq 17,52$$

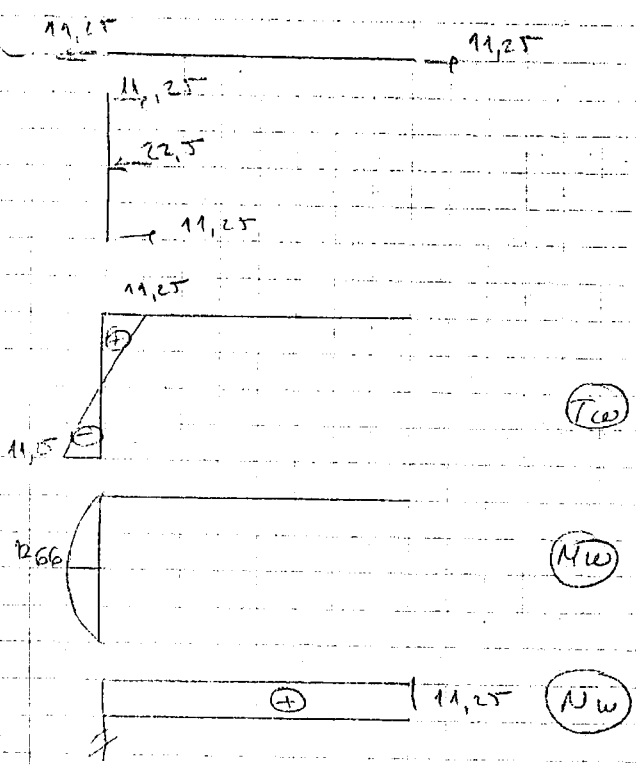
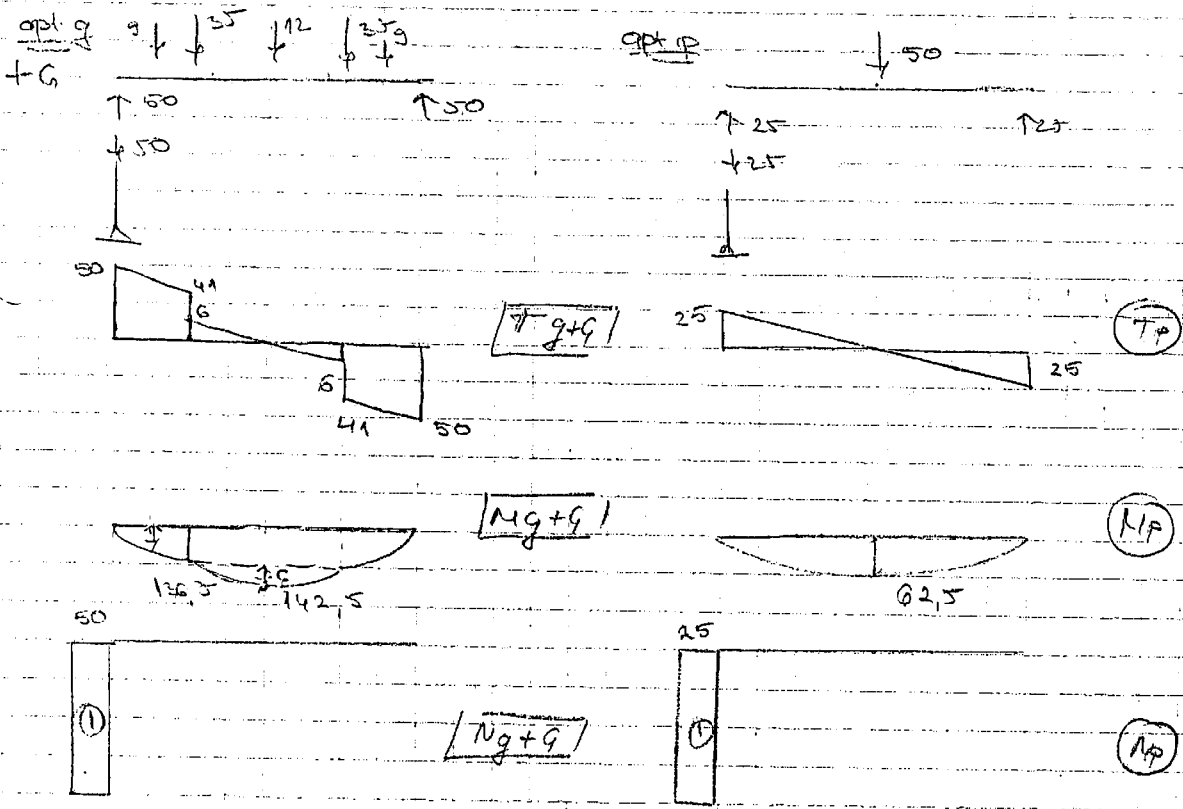
$$18,53 \leq 17,52 \quad \nabla \quad \Rightarrow \quad \underline{\text{veći profil}}$$

b) dimenzionirati cehtak prema dozvoljenom naprezanju. Za dat. presjek cehtaka
verifikovati nosnost prema dozvoljenom naprezanju objekta.

7



S 235
I sl. opt.



a) POS P

$T_{max} = 50 + 25 = 75 \text{ kN}$

$M_{max} = 142.5 + 62.5 = 205 \text{ kNm}$

$T_{00y} = 0$

a) pos 7 (obrat pažnje sta tačno dimenzionisani)

$$w_{dop} = \frac{h}{300} = 3,3 \text{ cm}$$

$$\left. \begin{array}{l} \sigma_{dop} = 16 \text{ kN/cm}^2 \\ \tau_{dop} = 9 \text{ kN/cm}^2 \end{array} \right\} \begin{array}{l} \sigma_{235} \\ \text{I sl} \end{array}$$

$$w_{pot} = \frac{M_{max}}{\sigma_d} = \frac{205 \cdot 100}{16} = 1281,25 \text{ cm}^3$$

$$A_{pot} = \frac{N_d}{\sigma_d} = \frac{11,25}{16} = 0,703 \text{ cm}^2$$

HEA 400

$$\begin{aligned} h &= 390 \text{ mm} \\ b_f &= 300 \text{ mm} \\ t_w &= 11 \text{ mm} \\ t_f &= 19 \text{ mm} \\ r &= 27 \text{ mm} \end{aligned}$$

$$\begin{aligned} A &= 159 \text{ cm}^2 \\ I_y &= 45070 \text{ cm}^4 \\ W_y &= 2310 \text{ cm}^3 \\ i_y &= 16,8 \text{ cm} \end{aligned}$$

$$\begin{aligned} I_z &= 8560 \text{ cm}^4 \\ W_z &= 571 \text{ cm}^3 \\ i_z &= 7,34 \text{ cm} \end{aligned}$$

$$S_y = 1280 \text{ cm}^3$$

* KONTROLA NAPONA KOD EKSCENTRIRANOG ZATEZANJA ELEMENATA IUS

$$\sigma_{max} = \frac{N_t}{A} + \frac{M_{max}}{W_y} = \frac{11,25}{159} + \frac{205 \cdot 100}{2310} = 8,94 \text{ kN/cm}^2 < 16 \text{ kN/cm}^2 \quad \checkmark$$

$$\tau_{max} = \frac{T_{max} \cdot S_y}{t_w \cdot I_y} = \frac{95 \cdot 1280}{11 \cdot 45070} = 1,94 \text{ kN/cm}^2 < 9 \text{ kN/cm}^2 \quad \checkmark$$

$$\tau_{max} = r_1 + r_2$$

$$r_1 = \frac{35 \cdot 300}{24 \cdot 21000 \cdot 45070} (3 \cdot 100^3 - 4 \cdot 30^3) = 1,22 \text{ cm}$$

$$r_{pomer} = 0,99$$

$$r_2 = \frac{5984}{3647 \cdot 21000 \cdot 45070} = 1,10 \text{ cm}$$

$$\left. \begin{array}{l} r_1 = 1,22 \text{ cm} \\ r_2 = 1,10 \text{ cm} \end{array} \right\} \tau_{max} = 2,32 \text{ cm} < 3,3 \text{ cm} \quad \checkmark$$

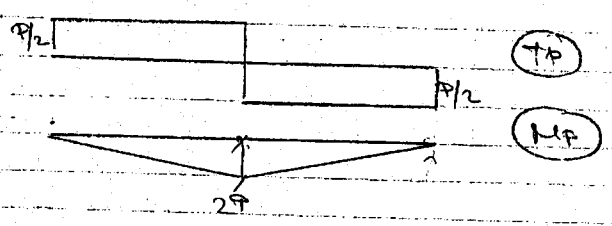
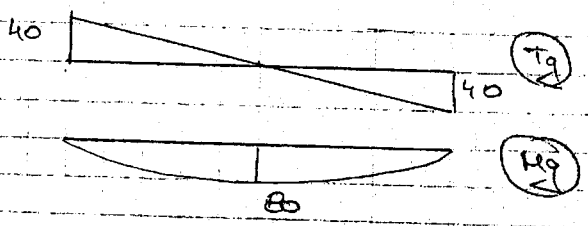
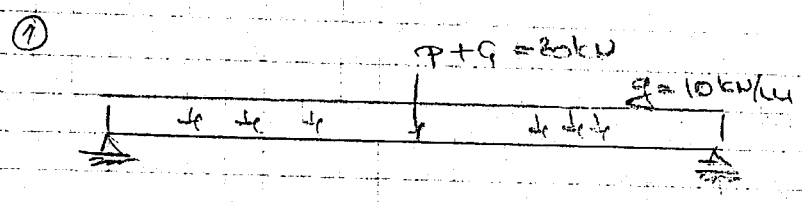
zavisno samo od toga sta h je prihvatilo ka nosacu (pogledaj 8. vezbu.)

* KONTROLA UPOREDNOG NAPONA [P 7-24]

$$\sigma_1 = \frac{M_{max}}{I_y} \frac{h-2 \cdot t_f}{2} + \frac{N_t}{A} = \frac{205 \cdot 100}{45070} \cdot \frac{390 - 2 \cdot 19}{2} + \frac{11,25}{159} = 8,08 \text{ kN/cm}^2$$

$$\tau_1 = \frac{T_{dop} \cdot S_{y,0}}{t_w \cdot I_y} = 0$$

$$\sigma_u = \sigma_1 = 8,08 \text{ kN/cm}^2 < 16 \text{ kN/cm}^2 \quad \checkmark$$



4. usvajalno IPE

5. ueteb4

① $w_{pot} = \frac{M_{max}}{\sigma_{dop}} = \frac{2P + 80kNm}{\sigma_{dop}} = P_1$

② $t_{max} = \frac{T_{max} \cdot S_y}{I_w \cdot I_y} \leq t_{dop} \Rightarrow \left(\frac{P}{2} + 40 \right) \cdot S_y \leq t_{dop} \cdot I_w \cdot I_y = P_2$

③ $\sigma_{\sigma} \leq \sigma_{dop} \Rightarrow \sigma_1^2 + 3\sigma_2^2 = \sigma_{dop}^2$
 $\left(\frac{M_{max}}{W_y} \cdot \frac{h - 2t_f}{2} \right)^2 + 3 \cdot \left(\frac{T_{dop}}{I_w} \cdot S_y \right)^2 = \sigma_{dop}^2 = P_3$

④ ugib

$f = \frac{5}{384} \frac{q l^4}{EI_y} + \frac{P}{48} \frac{l^3}{EI_y} \leq f_{dop} = P_4$

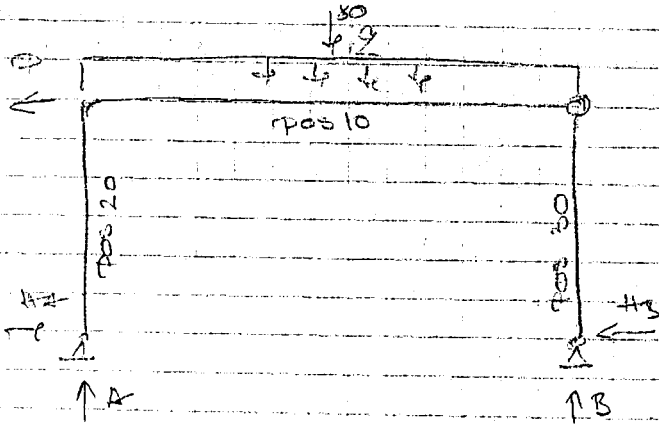
⑤ BTI.

$\frac{M_{max}}{W_y} < \frac{\sigma_{\sigma}}{V} = P_5$

4. usvajalno uinP

b) 4. ueteba KONTROLA NOSIVOSTI NA SVIJANJE I SMICANJE NOSIČA PREMA EC3

2



$P = 5 \text{ kN}$
 $Q = 20 \text{ kN}$

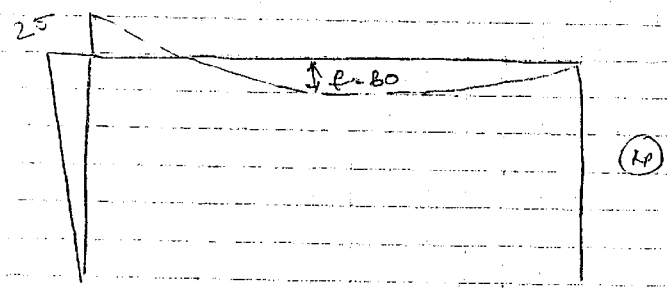
$H_A = -P$; $H_B = 0$

$M_A = 0 : 8 \cdot B + P \cdot 5 - 80 \cdot 4 = 0$

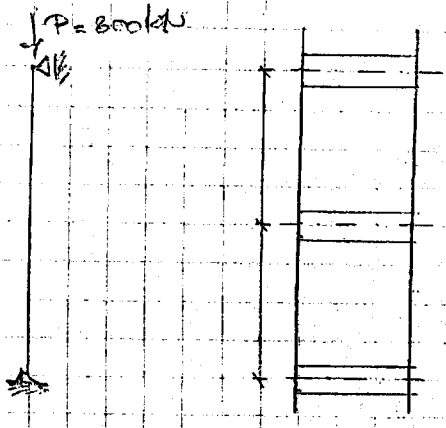
$B = \frac{320 - 25}{8} = 36,875$

$V_B = 0 : 80 \cdot 4 + 5 \cdot 5 - A \cdot 8 = 0$

$A = 43,125$



①



S 235
I sl. opt.

$\sigma_{dop} = 16 \text{ kN/cm}^2$
 $\lambda_0 = 92,3$

$P = 800 \text{ kN}$
 $h = 8 \text{ cm}$

L 80x80x8
 $A_1 = 15,5 \text{ cm}^2$
 $I_{y1} = I_{z1} = 116 \text{ cm}^4$
 $i_{y1} = i_{z1} = 2,74 \text{ cm}$
 $i_y = 1,76 \text{ cm}$

$a = 8 \text{ cm}$
 $h_y = 80 \text{ cm}$
 $e = 2,54 \text{ cm}$
 $I_y = 47,8 \text{ cm}^4$

$A = 4 \cdot A_1 = 62 \text{ cm}^2$
 $I_y = I_z = 4 \cdot (I_{y1} + A_1 \cdot (\frac{h_y}{2})^2)$
 $I_y = I_z = 56264 \text{ cm}^4$
 $i_y = i_z = \sqrt{\frac{I_y}{A}} = 30,12 \text{ cm}$

II KONTROLA STABILNOSTI NA IZVIJALJE

OKO NEVATERIJALNE OSE Y-Y (2-2)

• određivanje ekvivalentne uticost

$\lambda_y = \lambda_{y2} = \beta \cdot l = 1 \cdot 800 \text{ cm} = 800$
 $\lambda_y = \frac{\lambda_{y2}}{i_y} = \frac{800}{30,12} = 26,56$

$\lambda_e = \frac{a}{i_y} = \frac{80}{1,76} = 45,45$ • VITKOST SAMOSTALNOG ELEMENTA

$\lambda_e = \frac{a}{i_y}$ ako a i

$\lambda_{y,eq} = \lambda_{z,eq} = \sqrt{\lambda_y^2 + \frac{1}{2} \cdot \lambda_e^2} = \sqrt{26,56^2 + \frac{1}{2} \cdot 45,45^2} = 52,64$

za ramu str. 665.

• relativna ekvivalentna uticost

$\bar{\lambda}_{y,eq} = \bar{\lambda}_{z,eq} = \frac{\lambda_{y,eq}}{\lambda_0} = \frac{52,64}{92,3} = 0,567$

$\alpha = 0,489$ (kriiva izvijanja a)

$\beta_{z,eq} = 1 + \alpha (\bar{\lambda}_{z,eq} - 0,2) + \bar{\lambda}_{z,eq}^2$

$\beta_{z,eq} = 1 + 0,489 (0,567 - 0,2) + 0,567^2 = 1,501$

$\chi_{z,eq} = \frac{2}{(\beta_{z,eq} + \sqrt{\beta_{z,eq}^2 - 4 \cdot \bar{\lambda}_{z,eq}^2})} = \frac{2}{(1,501 + \sqrt{1,501^2 - 4 \cdot 0,567^2})} = 0,805$

* KONTROLA STABILNOSTI

$\sigma = \frac{N_d}{A} = \frac{800}{62} = 12,88 \text{ kN/cm}^2 < \sigma_{dop} = \chi_{z,eq} \cdot \sigma_{dop} = 0,805 \cdot 16 = 12,90 \text{ kN/cm}^2$

1) KONTROLA STABILNOSTI NA IZUJAJE SAMOSTALNOG ELEMENTA NA SREDINI VISINE VIŠEDELNOG STAPA

• određuje se u samostalnom elementu

$$N_{pe} = A \cdot f_y = 62 \cdot 24 =$$

$$N_{cr} = \frac{\bar{u}^2 \cdot E I_z}{L^2} = \frac{\bar{u}^2 \cdot 21000 \cdot 56264}{80^2} = 18220,89 \text{ kN}$$

$$S_y = 2 \frac{\bar{u}^2 E I_z}{a^2} \quad \text{— KRUŽOST VIŠEDELNOG STAPA NA SREDINI VISINE}$$

I_z — uov. inercije u odnosu na osu y-y str. 665.

$$S_y = 2 \frac{\bar{u}^2 \cdot 21000 \cdot 49,8}{80^2} = 3095,97 \text{ kN}$$

$$\frac{1}{N_{d,u}} = \frac{1}{N_{cr}} + \frac{1}{S_y} = \frac{1}{18220,89} + \frac{1}{3095,97} \Rightarrow N_{d,u} = 2646,32 \text{ kN}$$

$$M_y = \frac{N_c \cdot \rho_0}{1 - \nu \cdot N_c}$$

N_{c,u} — početna geometrijska imperfekcija
L/500

$$M_y = \frac{600 \cdot \frac{800}{500}}{1 - \frac{1,5 \cdot 600}{2646,32}} = 2342,00 \text{ kNm}$$

$$W_z = \frac{I_z}{y_{f, max}} = \frac{56264}{292,46} = 189,15 \text{ cm}^3$$

$$y_{f, max} = \frac{600}{2} = 292,46 \text{ cm} = 292,46 \text{ cm} \quad \text{e}$$

Y_{f, max} — rast. do kraja samostalnog elementa

$$N_1 = \frac{N}{\gamma} + \frac{M_z}{W_z} \cdot A_1 = \frac{600}{4} + \frac{2342,00}{189,15} \cdot 15,5 = 391,92 \text{ kN}$$

• uticost samostalnog elementa

$$\bar{\lambda} = \frac{\lambda_p}{\lambda_u} = \frac{45,45}{92,9} = 0,489$$

$$\beta_2 = 0,489 \Rightarrow \beta_2 = 1 + \alpha (\bar{\lambda} - 0,2) + \bar{\lambda}^2 = 1 + 0,489 (0,489 - 0,2) + 0,489^2 = 1,38$$

$$\mu_2 = \frac{2}{\beta_2 + \sqrt{\beta_2^2 - 4\bar{\lambda}^2}} = \frac{2}{1,38 + \sqrt{1,38^2 - 4 \cdot 0,489^2}} = 0,850$$

* kontrola stabilnosti

$$\sigma = \frac{N_1}{A_1} = \frac{391,92}{15,5} = 25,285 \text{ kN/cm}^2 < \sigma_{d, dop} = \mu_2 \cdot \sigma_{d, dop} = 0,850 \cdot 16 \text{ kN/cm}^2$$

[NIJE PROŠLO, ALI AJ DA ZAMISLIMO DA JESTE]

3) KONTROLA STABILNOSTI NA BUJANJE SAMOSTALNOG ELEKTA U KRAJNEM POLJU

• Osnogi u samostalnom elementu

$$N_f = \frac{N}{n} = \frac{800}{4} = 200 \text{ kN}$$

$$w_{\max V} = \frac{\sigma}{2} \cdot \frac{N_c \cdot w_0}{1 - \frac{N \cdot N_c}{N_{cr,0}}} = \frac{\sigma}{800} \cdot 2842 = \underline{9,197 \text{ kN}}$$

$$M_f = \frac{w_{\max V}}{r} \cdot \frac{a}{2} = \frac{9,197}{4} \cdot \frac{80}{2} = \underline{91,97 \text{ kNm}}$$

• KONTROLA NAPONA U SAMOSTALNOM ELEMENTU U KRAJNEM POLJU

$$\sigma = \frac{M}{W_{x,1}} + \frac{M_f}{W_f} = \frac{200}{15,5} + \frac{91,97}{13,3} = 19,82 \text{ kN/cm}^2 = 16,0 \text{ kN/cm}^2$$

(gle, nije preko!)

4) PROVERA SPOLNIH DIMENZI 667. str.

• Osnogi u spolnoj lamini

$$T = \frac{w_{\max V} \cdot a}{h_y} = \frac{9,197 \cdot 80}{600} = 12,26 \text{ kN}$$

$$M = \frac{w_{\max V} \cdot a}{r} = \frac{9,197 \cdot 80}{4} = 183,94 \text{ kNm}$$

* usvajaju se uzeti LHM 140x10x600

• KONTROLA NAPONA $\sigma = \frac{M}{W} = \frac{183,94}{\frac{141}{6}} = 5,63 \text{ kN/cm}^2$

$$\tau = \frac{T}{t \cdot h} = \frac{12,26}{1,14} = 0,878 \text{ kN/cm}^2$$

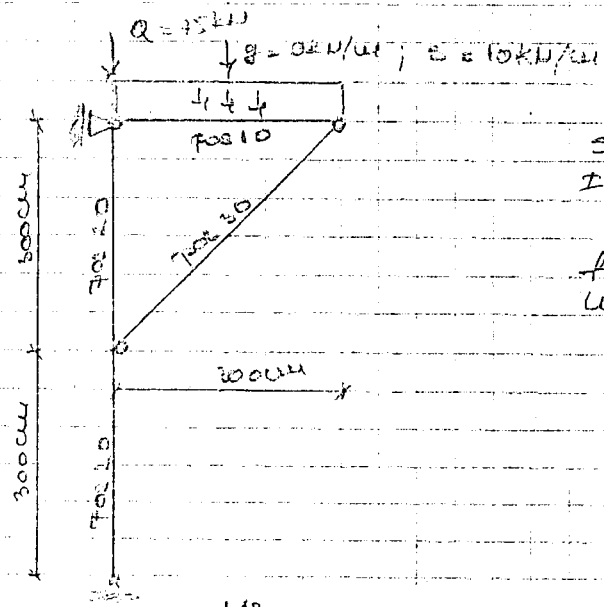
$$\sigma_u = \sqrt{\sigma^2 + 3 \cdot \tau^2} = \sqrt{5,63^2 + 3 \cdot 0,878^2} = \underline{5,83 \text{ kN/cm}^2} < \sigma_{\text{adm}}$$

* RAM

- 1) KONTROLA STABILNOSTI NA BUJANJE OKO MATERIJALNE OSE
- 2) KONTROLA STABILNOSTI NA BUJANJE OKO KENAMATERIJALNE OSE
- 3) KONTROLA NA BUJANJE SAMOSTALNOG EL. NA SREDNJI URINE VIŠEDIELNOG STAPA U KRAJNEM POLJU
- 4) -"-
- 5) PROVERA SPOLNIH DIMENZI

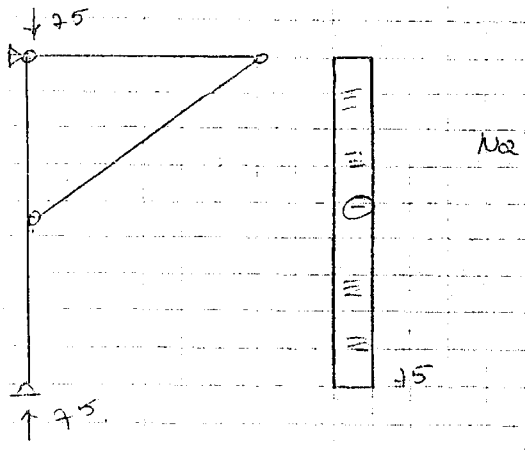
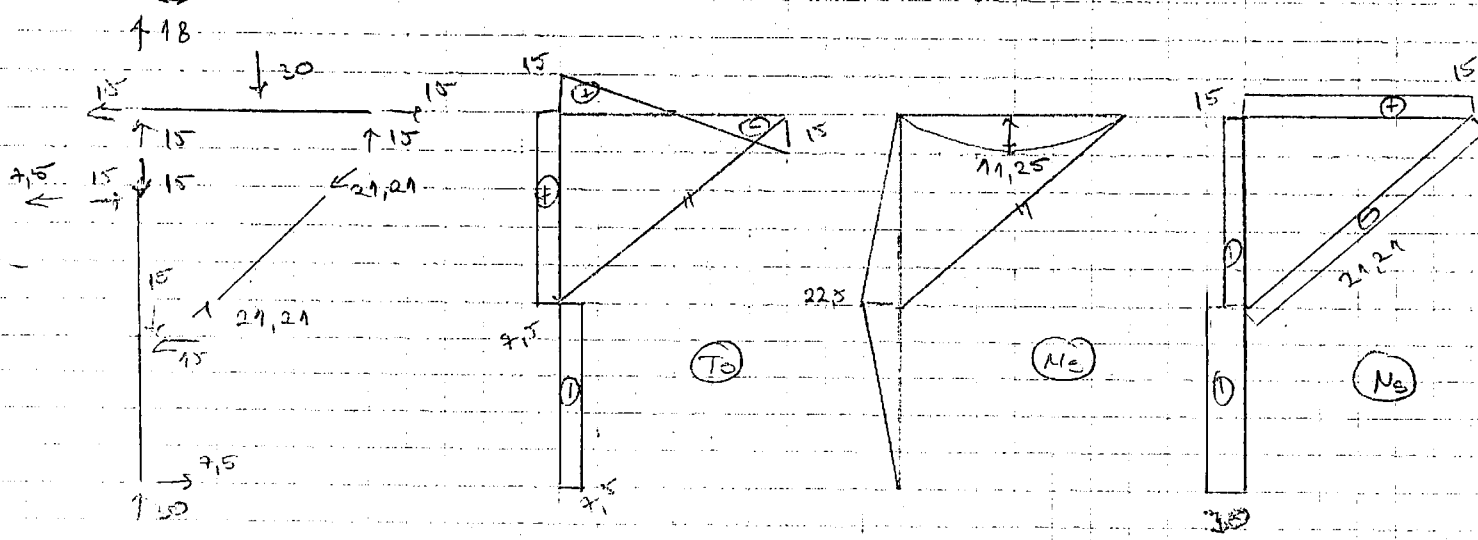
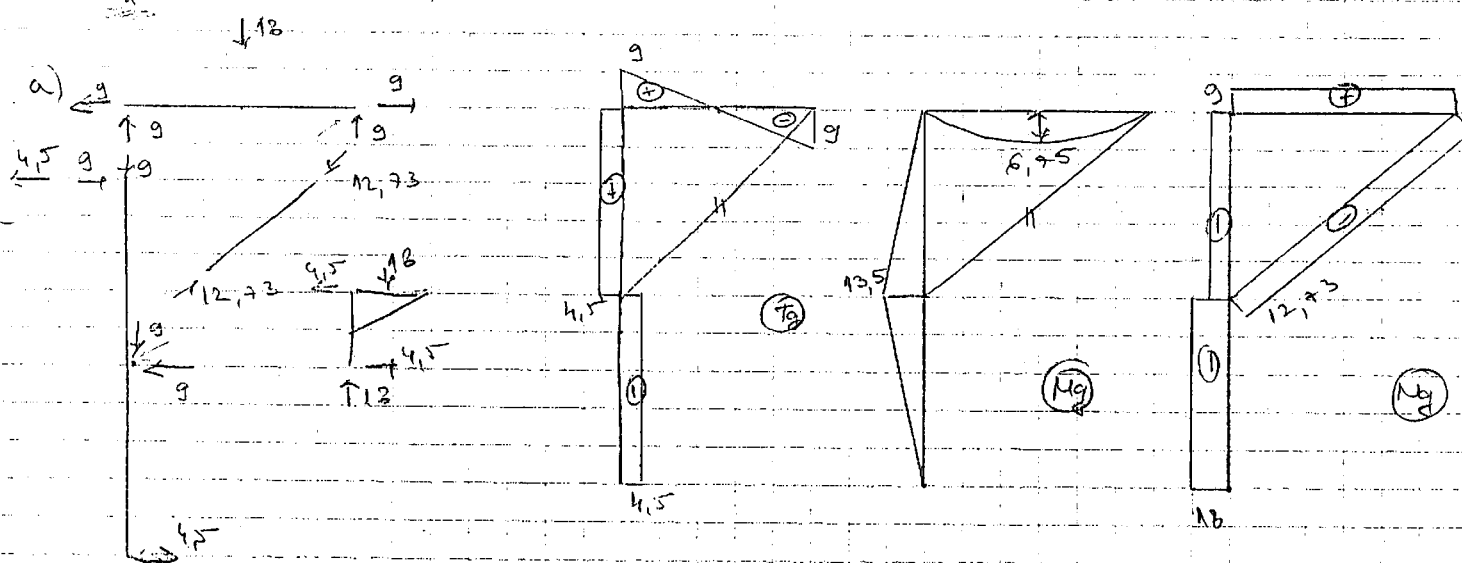
* REŠETRA (elaborat)

2



S_{225}
 I_{slopt}

$\sigma_{dop} = 16 \text{ kN/cm}^2$
 $f_{dop} = 1 \text{ cm}$
 $U_{dop} = 2,4 \text{ cm}$



a) pos 10 prava E03

$$S_{235} \quad \left. \begin{array}{l} \sigma_d = 16 \text{ kN/cm}^2 \\ \tau_d = 9 \text{ kN/cm}^2 \end{array} \right\}$$

$$f_{dp} = 1 \text{ cm}$$

$$W_{pot} = \frac{M_{max}}{\sigma_d} = \frac{18 \cdot 100}{16} = 112,5 \text{ cm}^3$$

$$A_{pot} = \frac{N_E}{\sigma_d} = \frac{24}{16} = 1,5 \text{ cm}^2$$

* usvajamo IPE 200

$$\begin{array}{lll} h = 200 \text{ mm} & I_y = 1940 \text{ cm}^4 & I_z = 112 \text{ cm}^4 \\ b_f = 100 \text{ mm} & W_y = 194 \text{ cm}^3 & W_z = 28,5 \text{ cm}^3 \\ t_w = 5,6 \text{ mm} & i_y = 8,26 \text{ cm} & i_z = 2,29 \text{ cm} \\ t_f = 8,5 \text{ mm} & L = 12 \text{ cm} & S_y = 110 \text{ cm}^3 \\ A = 28,5 \text{ cm}^2 & & \end{array}$$

* uncaj na vrhu nosa M

$$M_{ed} = 1,35 \cdot 69,5 + 1,5 \cdot 112,5 = 259,875 \text{ kNm}$$

$$N_{t,ed} = 1,35 \cdot 9 + 1,5 \cdot 15 = 34,65 \text{ kN}$$

$$\sigma_{x,ed} = \frac{N_{t,ed}}{A} + \frac{M_{ed}}{W_y}$$

* uncaj na dnu nosa N

$$N_{t,ed} = 34,65 \text{ kN}$$

$$\sigma_{x,ed} = \frac{34,65}{28,5} + \frac{259,875}{194} = 14,61 \leq \frac{f_y}{\gamma_{M0}} = \frac{23,5}{1} \text{ kN/cm}^2 \quad \checkmark$$

+ 4. vešba
(deprojeiciraj na, g/100...)

b) pos 30

$$A_{pot} = \frac{33,94}{16} = 2,2 \text{ cm}^2$$

030 □ 60x120

$$s = 7,1 \text{ cm}$$

$$A = 31,41 \text{ cm}^2$$

$$I_y = I_z = 661,7 \text{ cm}^4$$

$$W_y = W_z = 110,3 \text{ cm}^3$$

$$i_y = i_z = 4,59 \text{ cm}$$

$$e_y = e_z = 300 \cdot \sqrt{2} = 424,26 \text{ cm}$$

$$x_y = \frac{e_y}{4} = 92,43$$

$$\bar{\lambda}_y = 0,96 \quad \alpha = 0,489 \quad (\text{c knova})$$

$$\beta = 1 + \alpha (\pi - 0,2) + \bar{\lambda}^2 = 2,34$$

$$\mu = \frac{2}{\beta + \sqrt{\beta^2 - 4 \bar{\lambda}^2}} = 0,55$$

$$\sigma = \frac{N_E}{A} = \frac{33,94}{31,41} = 1,08 \text{ kN/cm}^2 \leq 0,55 \cdot 16 = 8,8 \text{ kN/cm}^2 \quad \checkmark$$

1) 1000 kg

$$U_{pot} = m \cdot g \cdot h = 3000 \cdot 10 \cdot 2.15 \text{ m} = 64.5 \text{ kJ}$$

$$U_{pot} = (m_1 + m_2) \cdot g \cdot h = (48 + 12) \cdot 10 \cdot 1 = 60 \text{ kJ}$$

1000 kg ...

→ Konstante Kraft

$$W_{max} = W_{pot} + \frac{\Delta W}{m_1} = 64.5 \text{ kJ} + \frac{16}{16} = 65.5 \text{ kJ}$$

$$T_{max} = T_{pot} \cdot \frac{m_1 + m_2}{m_1} = 60 \text{ kJ} \cdot \frac{60}{16} = 225 \text{ kJ}$$

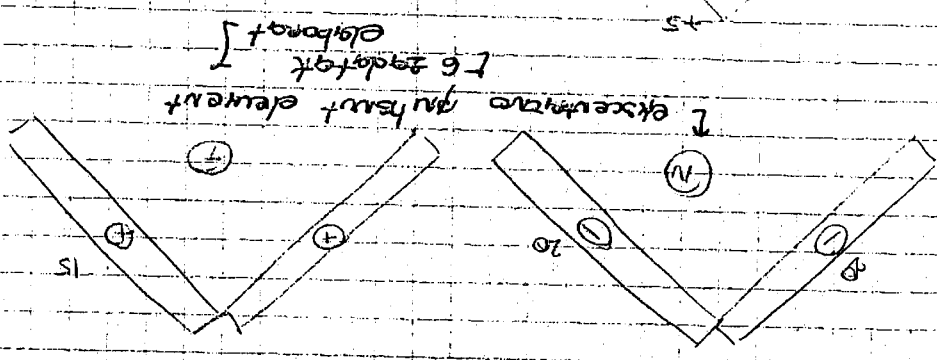
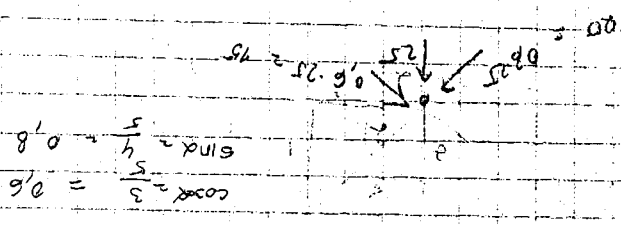
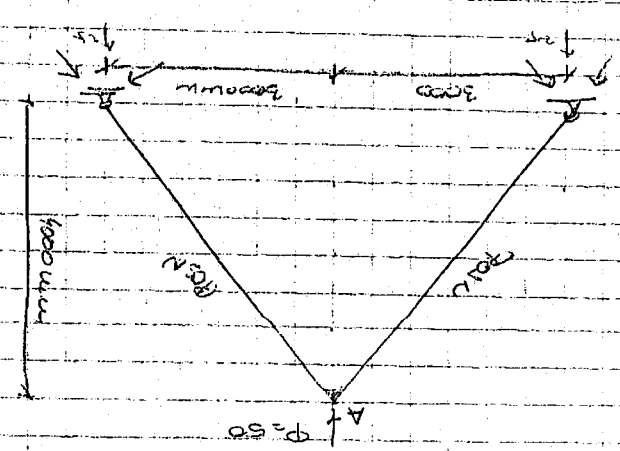
$$W_{pot} = m \cdot g \cdot h = 16 \cdot 10 \cdot 1 = 160 \text{ J}$$

$$W_{pot} = (m_1 + m_2) \cdot g \cdot h = (48 + 12) \cdot 10 \cdot 1 = 600 \text{ J}$$

→ Konstante Kraft & konstante Beschleunigung & konstante Elementar K...

→ Konstante Kraft & konstante Beschleunigung & konstante Elementar K...

Diketahui: $P = 50 \text{ kN}$. Untuk mencari gaya reaksi di titik A dan B, kita gunakan hukum kesetimbangan. Untuk mencari gaya reaksi di titik A, kita gunakan hukum kesetimbangan momen di titik B. Untuk mencari gaya reaksi di titik B, kita gunakan hukum kesetimbangan momen di titik A.



$5235 \left\{ \begin{array}{l} \text{Dip} = 16 \text{ kN/cm} \\ \text{Dip} = 9 \text{ kN/cm} \end{array} \right.$
 Isal

$$\text{Momen} = \frac{1}{2} \cdot b \cdot \text{Dip} = \frac{1}{2} \cdot 468 \cdot 15 \text{ cm} = 3510 \text{ cm}^2$$

* Persegi panjang selip

$A = 53.8 \text{ cm}^2$
 $W_y = 552 \text{ cm}^3$
 $I_y = 8260 \text{ cm}^4$
 $S_y = 314 \text{ cm}^3$
 $t = 15 \text{ mm}$
 $t_f = 10 \text{ mm}$
 $t_w = 7.1 \text{ mm}$
 $b = 170 \text{ mm}$
 $h = 300 \text{ mm}$

Ke KURDOLA KURDOLA

- Plat selip

$$\text{Momen} = \frac{M_{max}}{N} + \frac{W_y}{A} = \frac{2510}{50} + \frac{552}{53.8} = 50.2 + 10.27 = 60.47 \text{ cm}^2$$

$$\text{Momen} = \frac{M_{max}}{S_y} = \frac{0.1 \cdot 8260}{314} = 0.263 \text{ cm}^2 > \text{Dip} = 9 \text{ kN/cm}^2$$

Handwritten text at the top of the page, possibly a title or header, including the word "KURVA" and some illegible characters.

$$\sigma_{11} = \frac{M}{A}$$

$$\sigma_{11} = \frac{\sigma_{11}}{\sigma_{11}}$$

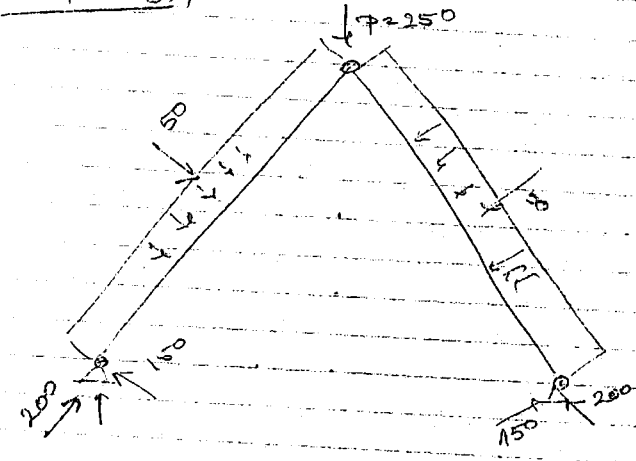
$$\sigma_{11} = \frac{M}{W}$$

Handwritten text below the first equation, possibly a label or unit.

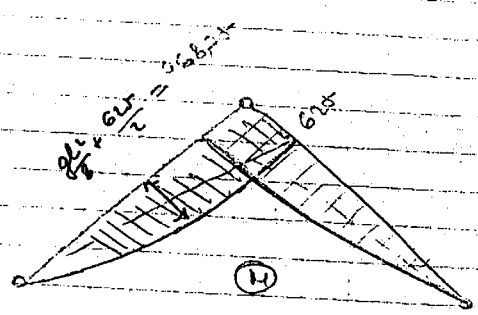
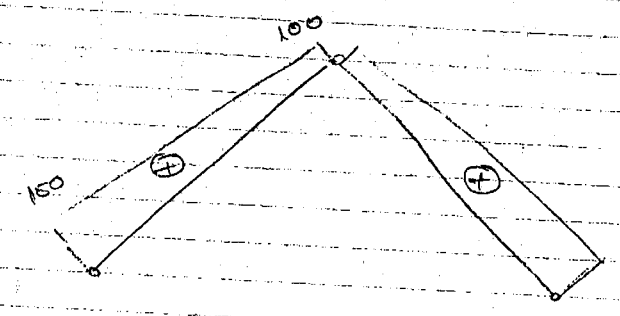
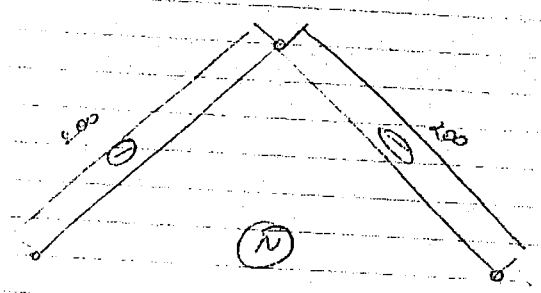
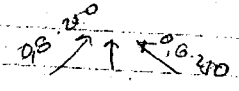
Handwritten text and symbols, including a triangle and some illegible characters.

Handwritten text and symbols, including a triangle and some illegible characters.

15.4.100G.1



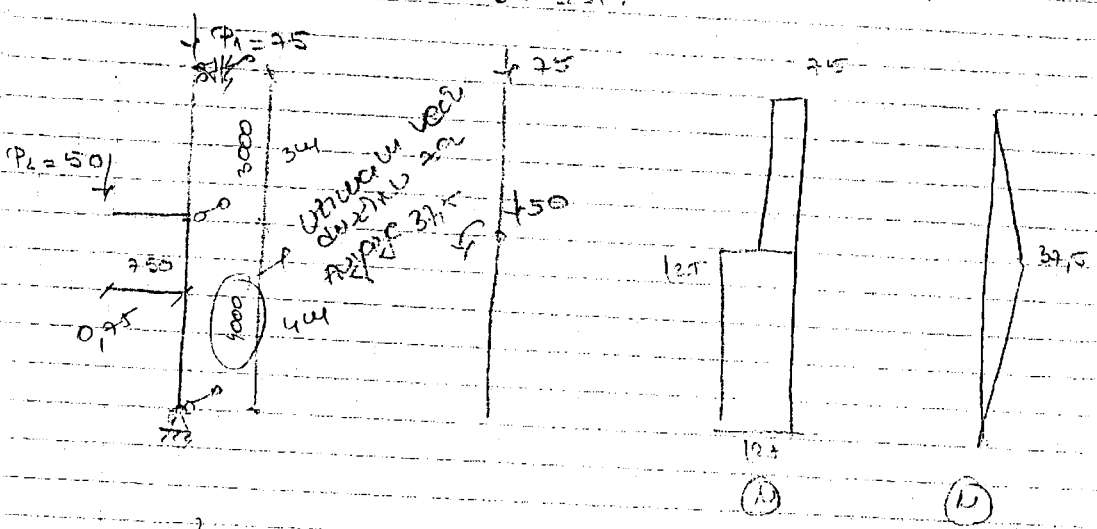
- Trake boony padeisrang
 as rad oslounun,
 d doun =/ $e_x =$
 $e_y =$



EXSCENTRICUS PRINSUT ELEMENT
 G. VERBA

Zadatok 2

Dimenzionirah stub puzovan na skri. Stub je opterezen koncentracijom silo na vrhu ($P_1 = 25 \text{ kN}$) i na osnovu ($P_2 = 50 \text{ kN}$). Stub je lociran u odnosu na osu udesi udesa sa korolom, to je projekt stuba udesi udesa udesi IPE 270h1. S235 JSL.



S235 / $\sigma_{dop} = 16 \text{ kN/cm}^2$

$$W_{pot} = \frac{M_{max}}{\sigma_{dop}} = \frac{37,5 \cdot 100}{16} = 234,375 \text{ cm}^3 = \text{predp. IPE 270}$$

$$\sigma = \frac{N}{A} + \frac{M}{W_y} = \frac{125}{45,9} + \frac{37,5 \cdot 100}{423} = 91,46 < \sigma_{dop} = 16 \text{ kN/cm}^2 \text{ w.}$$

Usporedno IPE 270 $\Rightarrow I_y = 5790 \text{ cm}^4, W_y = 423 \text{ cm}^3$

Element < σ_{dop} w
Nar = 0

+ KONTROLA EKSCENTRICNO PRIMENJENOG ELEMENTA $| k_n \cdot \sigma_n + \beta \cdot k_{ny} \cdot \sigma_{ny} \leq \sigma_{dop} |$

$$\sigma_n = \frac{N}{A} = \frac{125}{45,9} = 2,72$$

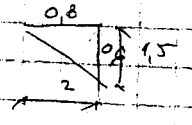
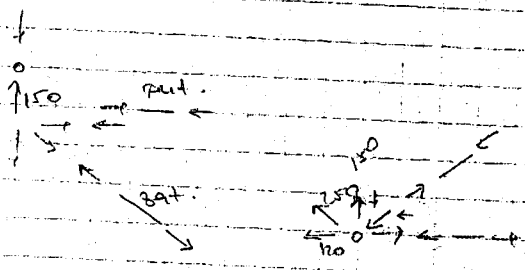
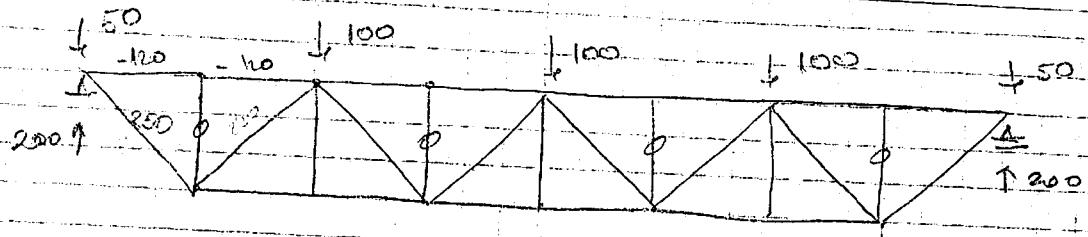
$$\sigma_{ny} = \frac{M}{W_y} = \frac{37,5 \cdot 100}{423} = 8,87$$

$$\sigma_{ny} = \frac{\sigma_n}{\sigma_{dop}} = \frac{2,72}{16} = 0,17$$

- Utkost
- ~ 0 da ubu $\lambda_{y1} = \frac{e_{y1}}{i_y} = \frac{e \cdot 1}{11,2} = \frac{700}{11,2} = 62,5$
- ~ 1200 $\lambda_{y2} = \frac{e_{y2}}{i_y} = \frac{e \cdot 2}{11,2} = \frac{1400}{11,2} = 125$

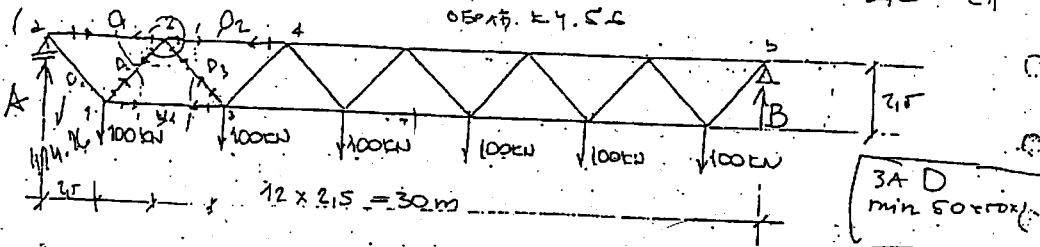
Ali treba da se udesi korolom temporo multipaje $e_{y1} = ? ; e_{y2} = ?$

16. Sept. 2002



3) 2L 60x60

00361 (Ica. om) $\sigma_{dop} = 16 \frac{KN}{cm^2}$
 05P15. K.Y. C.C



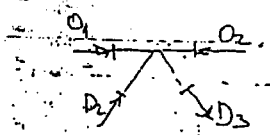
A = 300 kN B = 300 kN ✓

$\sum M(A) = 0 \quad 2.5 \cdot A + O_1 \cdot 2.5 = 0 \Rightarrow O_1 = -A \quad \boxed{O_1 = -300 \text{ kN}}$ притискат

$\sum M(B) = 0 \quad A \cdot 5 - 100 \cdot 2.5 - U_1 \cdot 2.5 = 0 \Rightarrow \boxed{U_1 = 500 \text{ kN}}$

$\sum M(A) = 0 \quad 100 \cdot 2.5 - D_2 \cdot 2.5 - U_1 \cdot 2.5 = 0 \Rightarrow \boxed{D_2 = -282.84 \text{ kN}}$

$\sum F_y = 0 \Rightarrow \boxed{D_3 = -D_2 = 282.84 \text{ kN}}$



$O_2 = O_1 + (D_2 + D_3) \frac{1}{2}$
 $= -O_1 + D_2 \frac{1}{2}$
 $\boxed{O_2 = 700 \text{ kN}}$ — притискат

ЗАТЕГНАТА ДИЈАГОНАЛА (D_3) — JL
 ПРИТИСНАТА ДИЈАГОНАЛА (D_2) — JL
 ПОЈАСНИ ШТАПОВИ ВАНУМ И ПРОТИВ

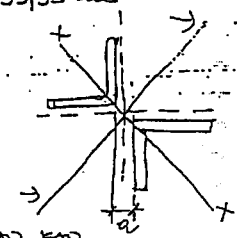
1) ЗАТЕГНАТА ДИЈАГОНАЛА $D_3 = 282.84 \text{ kN}$ — JL

$\frac{D_3}{2 A_{\text{прот}}} \leq \sigma_{dop} \Rightarrow A_{\text{прот}} \geq \frac{D_3}{2 \cdot \sigma_{dop}} = \frac{282.84}{2 \cdot 16} = 8.84 \text{ cm}^2$

УСЛАВА СС — 2L 60x60x8 (A = 9.03 cm² — погрешна избор на профил)

2) ПРИТИСНАТА ДИЈАГОНАЛА $D_2 = 282.84 \text{ kN}$ $l_2 = 353.55 \text{ cm}$

$A_{\text{прот}} \geq \frac{D_2}{2 \cdot \sigma_{dop}} = \frac{282.84}{2 \cdot 16} = 8.84 \text{ cm}^2$
 $\Rightarrow \boxed{2L 100 \times 100 \times 10 \quad (A_n = 19.2 \text{ cm}^2)}$



$z_1 = 1.95$ (ПОЛЗПР. ИЛИ СМЕРСТАЛНОГ ЕЛ.)

$I_x = 560$

$l_x = 318.2$

ДА БИ СЕ ШТАП ГРЕСНИЛО КАО
 ЧЕЛНОСЛАНИ ТРЕБА ДА ДАЖИ $\alpha \leq 70 \alpha_1 = 81.2$

$I_y = ?$ ШТА ДА
 $\alpha \leq 70$

$\frac{l_2}{r_2} = 88.39$
 $\frac{l_2}{r_1} = 117.87$

$\beta = 0.9 \rightarrow$ ЗА — И МЕРОДАНУМ ДЕ l_x

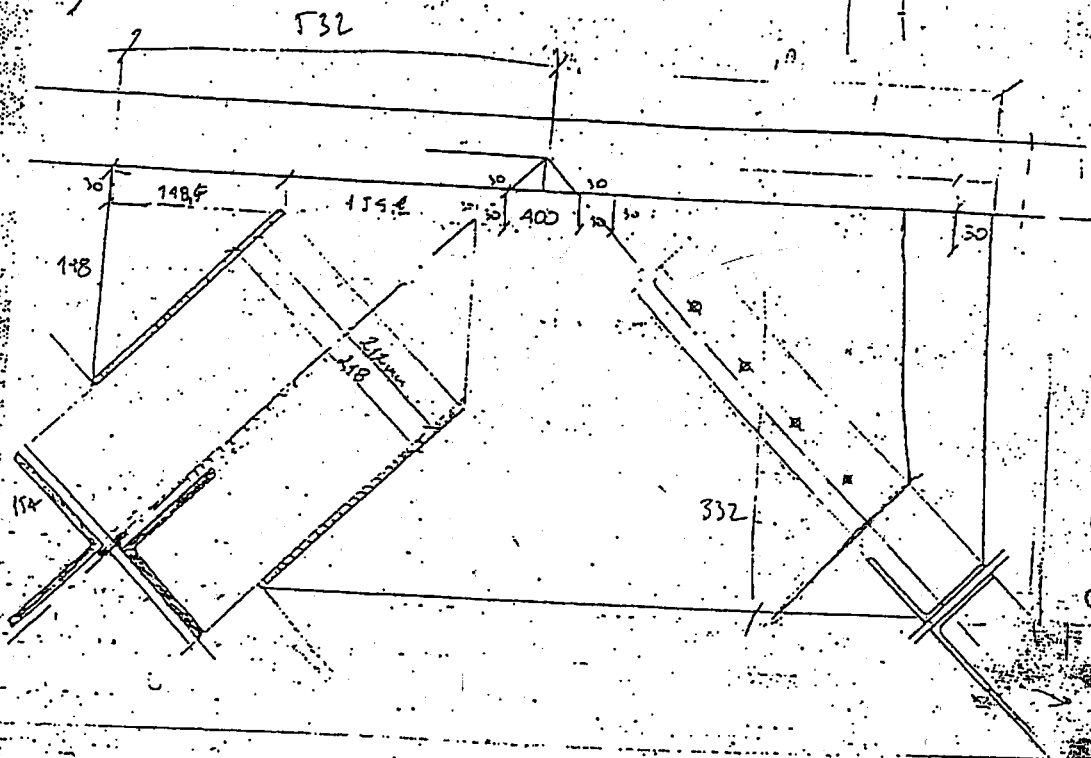
$l_x = \beta \cdot l_2 = 0.9 \cdot 353.55 = 318.20 \text{ cm}$

$\lambda_{lx} = \frac{l_x}{r_x} = \frac{318.20}{318.2} = 1.0$

$\lambda = \frac{\lambda_{lx}}{\lambda_v} = \frac{1.0}{0.93} = 1.07 > 0.2$

$\lambda_v = 92.9 \quad \text{ЗА } t \leq 40 \text{ mm}$
 $\sigma_v = 24 \frac{KN}{cm^2}$
 $d = 0.189$ (КРИВА С)

$\beta = 1 + \alpha (\lambda - 0.2) + \lambda^2 = 2.15$



$$l_s = 210 \text{ mm}$$

УДОПУА. АУМ

$$\sigma_{\text{вн}} = \frac{Z}{(b_{\text{ef}} - d_n) \cdot t_{\text{вн}}} \leq \sigma_{\text{доп}}$$

$$= \frac{282,84}{(34,85 - 2,1) \cdot 1,2} = 7,10 < \sigma_{\text{доп}} = 16$$

$$b_{\text{ef}} = b + 2 \cdot l_s \cdot t_{30} = 10,6 + 2 \cdot 21 \cdot t_{30} = 34,85$$

Z =

=

$$\lambda > 0,2 \Rightarrow \kappa = \frac{2}{1 + \sqrt{1 - 4\lambda^2}} = 0,60$$

$$\sigma_{1,dop} = \kappa \cdot \frac{\sigma_y}{\gamma} = 0,65 \cdot \frac{24}{1,5} = 9,62 \quad (\gamma = 1,5 \text{ SA OCHUVANO ONT.})$$

$$\sigma = \frac{D_3}{2A_{Ausv.}} = \frac{282,84}{2 \cdot 19,2} = 7,36 < \sigma_{1,dop}$$

УСВАЖА СЕ 2L 100 x 100 x 10 ($A_1 = 19,2 \text{ cm}^2$)

3) ПОДАКТИ ВИТАРОВА (ПРЕНА ВЕДОЈ СИЛИ $\Rightarrow O_2 = 700 \text{ KN}$)

ВРАТТИ I ПРОФИЛ

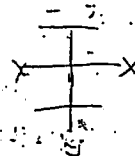
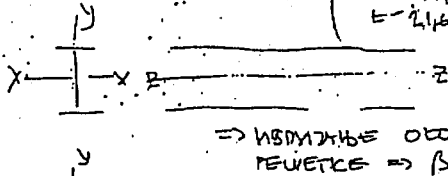
ПРВТИСАК

$l_2 = 500$

$$A_{pot} \geq \frac{O_2}{\sigma_{dop}} = \frac{700}{46} = 43,75 \text{ cm}^2$$

$$\Rightarrow I 400 \quad \left\{ \begin{array}{l} A = 118 \\ l_x = 400 \\ b = 157 \\ i_x = 44,4 \\ e = 2,16 \end{array} \right.$$

$$\left\{ \begin{array}{l} i_x = 15,0 \\ i_y = 10,4 \end{array} \right.$$



\Rightarrow ИСПИТАЊЕ ОДО X ОДЕ ЈЕ У РАВНИ РЕВЕТКЕ $\Rightarrow \beta_x = 1$

\Rightarrow ИСПИТАЊЕ ОДО Y ОДЕ ЈЕ У ПРАВО АА РАВНИ РЕВ. $\Rightarrow \beta_y = 0,8$

$$l_{ix} = \beta_x \cdot l_2 = 1 \cdot 500 \text{ cm} = 500 \text{ cm}$$

$$l_{iy} = \beta_y \cdot l_2 = 0,8 \cdot 500 \text{ cm} = 400 \text{ cm}$$

$$\lambda_{ix} = \frac{l_{ix}}{i_x} = \frac{500}{15,7} = 31,85$$

$$\lambda_{iy} = \frac{l_{iy}}{i_y} = \frac{400}{10,4} = 38,46$$

$$\lambda = \max\{\lambda_{ix}, \lambda_{iy}\} = 38,46$$

$$\lambda_v = 92,9$$

$$I \Rightarrow \frac{b}{t} = \dots \rightarrow 1,2 \quad A \cdot t < 40 \text{ mm} \Rightarrow \begin{array}{l} (x-x) \Rightarrow \text{КРИВА A} \\ (y-y) \Rightarrow \text{КРИВА B} \end{array}$$

$$\alpha = 0,339 \quad 159,74 \quad 1,72$$

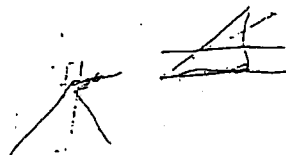
$$\bar{\lambda} = \frac{\lambda}{\lambda_v} = \frac{38,46}{92,9} = 0,414 > 0,2$$

$$\beta = 1 + \alpha (\bar{\lambda} - 0,2) + \bar{\lambda}^2 = 3,25$$

$$\kappa = \frac{0,65}{\beta} = 0,273 \Rightarrow \sigma_{1,dop} = \frac{24}{1,5} \cdot 0,273 = 4,37$$

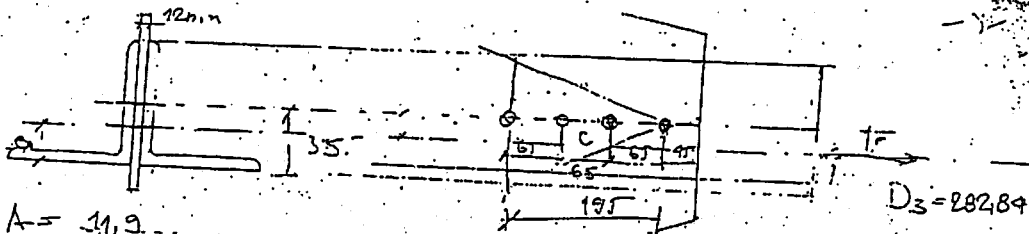
$$\sigma = \frac{O_2}{A_{Ausv.}} = \frac{700}{118} = 5,93 < \sigma_{1,dop}$$

ИЗБОРИТИ I 450!



ОБРАБЕЖЕНА ЗАВРТВЕДА К.Ч. 5.6

2L 70x70x9



$A = 11,2$

$\alpha = 20$

$\beta = 5$

$c_{\alpha} = c_{\beta} = 1,05 = 20,5 \text{ mm}$

$\sigma_{b, dop} = 32 \frac{\text{KN}}{\text{cm}^2}$

$\tau_{dop} = 17,5 \frac{\text{KN}}{\text{cm}^2}$

$C = 35 - 2,05 = 14,5$

$d_{max} = \sqrt{5 \cdot t_{s, min} - 0,2} = \sqrt{5 \cdot 0,8 - 0,2} = 1,8 \text{ cm} \quad (\pm 2 \text{ mm})$

УСБ. M20xL - 5.6

$A_{neto} = A - 2 \cdot 1 \cdot 0,8 = 10,4 \text{ cm}^2$

$\Rightarrow \sigma = \frac{D_3}{2 A_{neto}} = 14,5 \frac{\text{KN}}{\text{cm}^2} < 16 \frac{\text{KN}}{\text{cm}^2}$

$F = m \cdot n \cdot \left\{ \begin{aligned} F_s &= m \cdot d_1 \cdot \frac{\sigma_{dop}}{4} = 2 \cdot \frac{21^2}{4} \cdot 17,5 = 121,23 \text{ KN} \\ F_b &= d_1 \cdot \Sigma t_{s, min} \cdot \tau_{dop} = 2 \cdot 1 \cdot 1,2 \cdot 32 = 80,64 \text{ KN} \end{aligned} \right.$

$F = 80,64 \text{ KN}$

$n = \frac{D_3}{F} = \frac{282,84}{80,64} = 3,51$

$n = 4$

Сила у жоуа: закрити

$E_2 = \frac{D_3}{n} = 70,71$

$F_n = \frac{M \cdot r_{max}}{\Sigma r_c^2}$

$F_R = \sqrt{20,71^2 + 189,28^2}$

$= 190,71$

$= \frac{D_3 \cdot C \cdot \frac{3}{2} \cdot 6,5}{2 \cdot (\frac{6,5}{2})^2 + 2 \cdot (\frac{3}{2} \cdot 4)^2}$

$= \frac{282,84 \cdot 14,5 \cdot \frac{3}{2} \cdot 6,5}{2 \cdot \frac{42,25}{4} + 2 \cdot 36}$

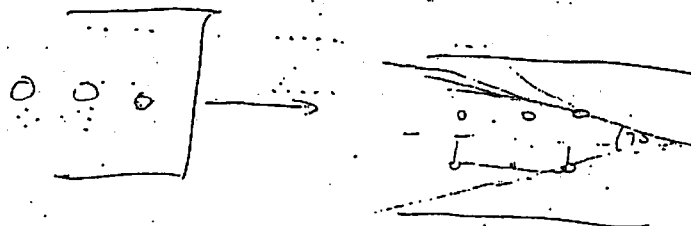
$= 189,28$

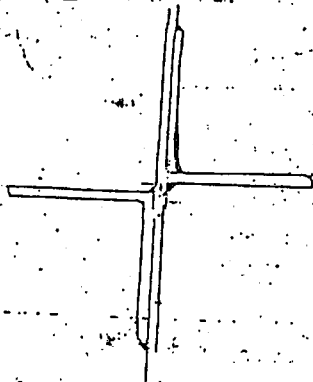
$= 189,28$

$5 \cdot 20,71 < 190,71 < F$

$b_{cf} = 195 \cdot \sin 45^\circ = 137,5 \text{ mm} = 13,75 \text{ cm}$

(113)





$$V_{II} = \frac{D}{4As} = \frac{D}{4al} \quad \sigma_{II} = \sigma_{доп} = 12 \quad l = l_s - 2a$$

$$\Rightarrow \frac{282,24}{4 \cdot 0,3 \cdot l} \leq 12 \quad \sigma_s = 0,3 \text{ mm}$$

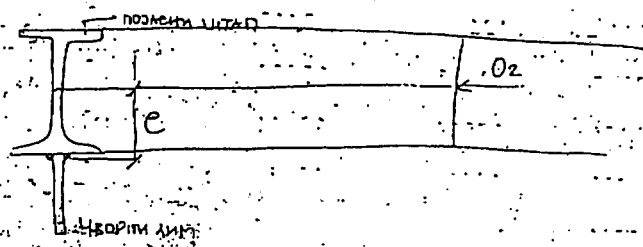
$$e > \frac{282,24}{4 \cdot 0,3 \cdot 12} = 19,64 \text{ см}$$

$$l_s = 0,6 > 19,64$$

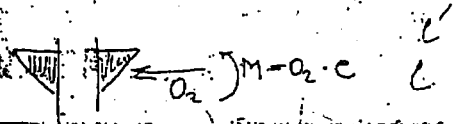
$$l_s > 0,2 \cdot 24$$

$$l_s = 11 \text{ см}$$

3) Доста ли бетонна мина за дожену мина у савременој пракси



$\sigma_s = 0,3 \text{ mm}$
+ ЧАСТ



$$n = \frac{M}{2W_s} = \frac{\sigma_s e}{2 \cdot \frac{al^2}{6}} = \frac{3\sigma_s e}{al^2}$$

$$\sigma_{II} = \sqrt{n^2 + V_{II}^2} \leq \sigma_{доп}$$

$$V_{II} = \frac{0,2}{2ae}$$

$$\left(\frac{3 \cdot 0,2 \cdot e}{al^2}\right)^2 + \left(\frac{0,2}{2ae}\right)^2 \leq 12$$

$$\sqrt{\frac{9 \cdot 0,2^2 \cdot e^2}{a^2 l^4} + \frac{0,2^2}{4a^2 e^2}} \leq 12$$

$$n = \frac{2 \cdot 0 \cdot e}{2 \cdot a l^2}$$

$$\frac{0,2}{al} \sqrt{\frac{9e^2}{e^2} + \frac{1}{4}} \leq 12$$

$$n = \frac{6 \cdot 20 \cdot e}{2al^2}$$

$$\frac{0,2}{2al} \sqrt{\left(\frac{6 \cdot 20}{e}\right)^2 + 1} \leq 12$$

$$V_{II} = \frac{20}{2ae}$$

$$\frac{700}{2 \cdot 0,3 \cdot e} \sqrt{\left(\frac{6 \cdot 20}{e}\right)^2 + 1} \leq 12$$

$$\frac{700}{2 \cdot 0,3 \cdot e} \sqrt{\left(\frac{720}{e}\right)^2 + 1} \leq 12$$

$l = 140 \rightarrow \sigma = 10,57$
 $l = 135 \rightarrow \sigma = 10,76$

$$l = 135 \text{ см}$$

$$l \geq 136 \text{ см}$$